

OCT 4 – 6, 2015 • Queens University • Kingston, ON Canada

*Challenges and Innovations in Tunnelling*

## Tunnelling Instrumentation – Support Design, Optimization and Testing

Nicholas Vlachopoulos, PhD, CD, PEng, PE

Associate Professor

Civil Engineering Department

Royal Military College of Canada

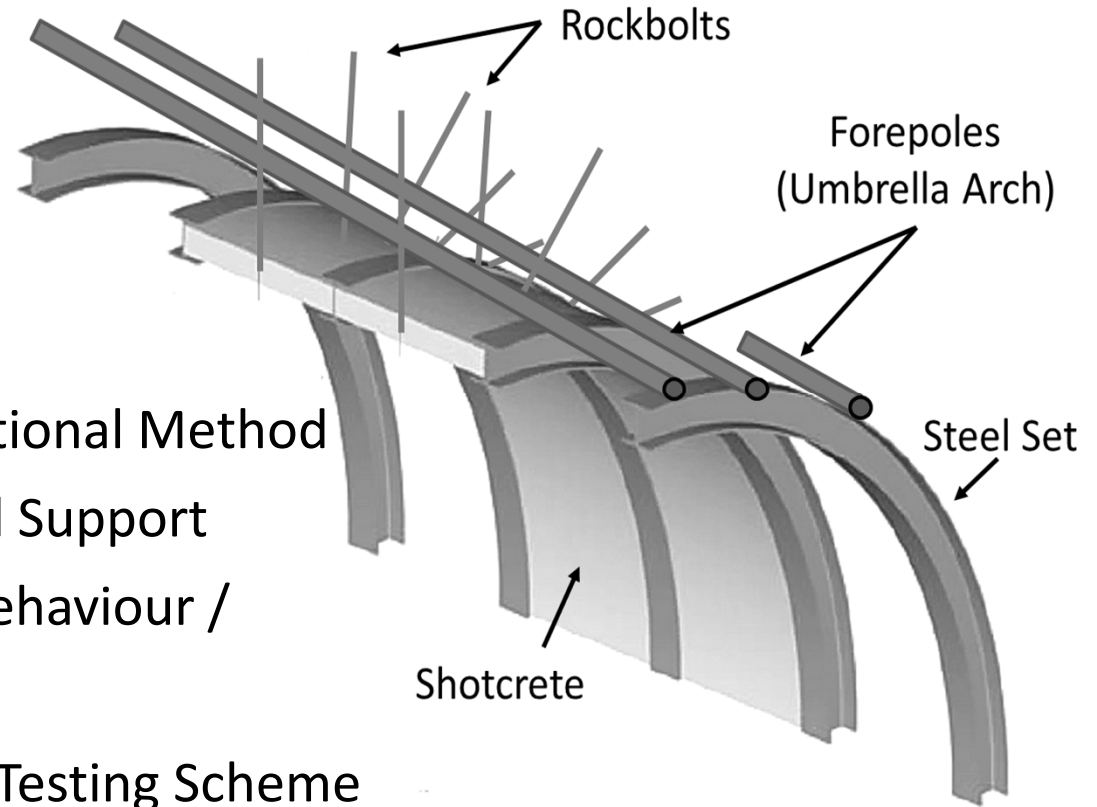
04 October 2015

Tunnelling Association of Canada  
Association Canadienne Des Tunnels



# Outline

1. Introduction
2. Instrumentation & Observational Method
3. Design of Temporary Tunnel Support
4. Factors Affecting Support Behaviour / Mechanisms
5. Instrumentation & Physical Testing Scheme
  - a. Outline of testing
  - b. Use of fibre optics
6. Selected Results
7. Conclusions / Questions / Discussion



# Observational Method

- Equations do not yield exact behaviour of site conditions
  - Results in over-conservative design
- The observational method employs instrumentation to predict actual site conditions
  - Leads to modification of original design to suit actual ground conditions



# Observational Method

- Common concerns in field monitoring operations:
  - Too many programs are based on the number of instruments to be used rather than on the questions to be answered
  - Sophistication and Automation are substituted for patient proof testing of equipment under field conditions



# Steps to Planning an Instrumentation Plan

- Engineers tend to proceed by first selecting instruments and taking data and then wondering what to do with the data. This is a bad idea. Instead a series of steps should be completed.
- *“We need to carry out a vast amount of observational work, but what we do should be done for a purpose and be done well”.*

Peck (1972)



# Steps to Planning an Instrumentation Plan

- There are 20 Steps to completing a proper instrumentation plan.
- Each step is important...

“a monitoring program is a chain with many potential weak links and breaks down with greater facility and frequency than most other tasks in geotechnical engineering”.

Franklin (1977)



# Instrumentation and Monitoring

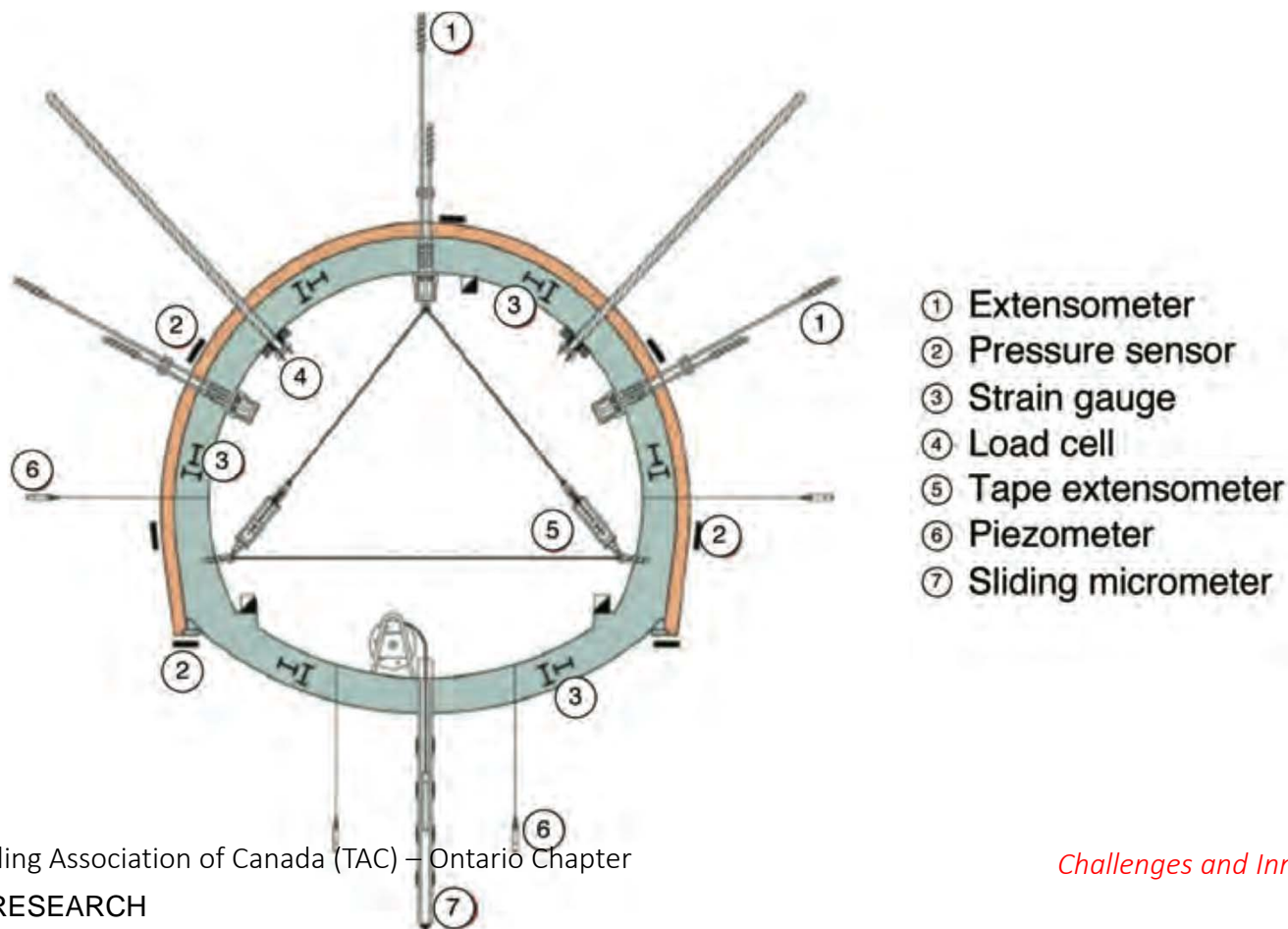
## Objective:

- Obtain information on ground response to tunneling
- Provide construction control
- Verify design parameters and models
- Measure performance of the lining during and after construction
- Monitor impact on the surrounding environments
- Optimization of the design and execution of safe tunneling works



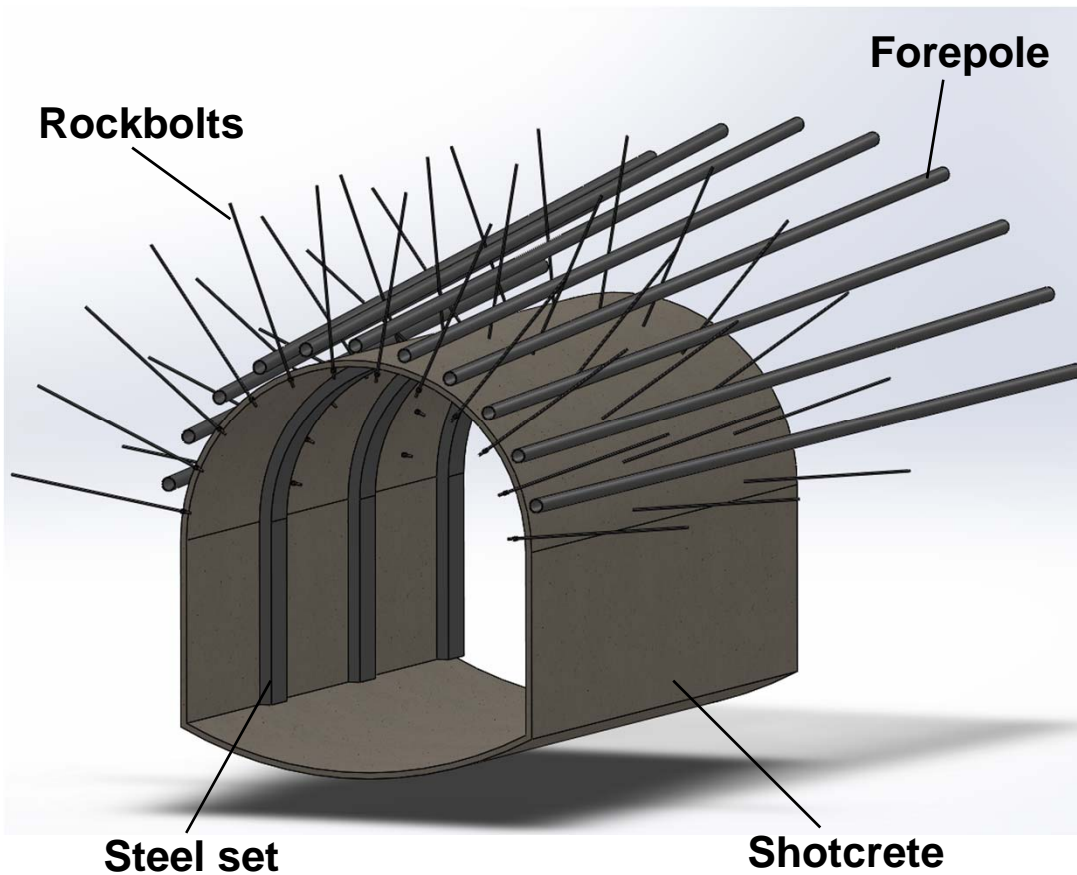
# Instrumentation and Monitoring

Generally instruments employed in tunnel monitoring are as follows:





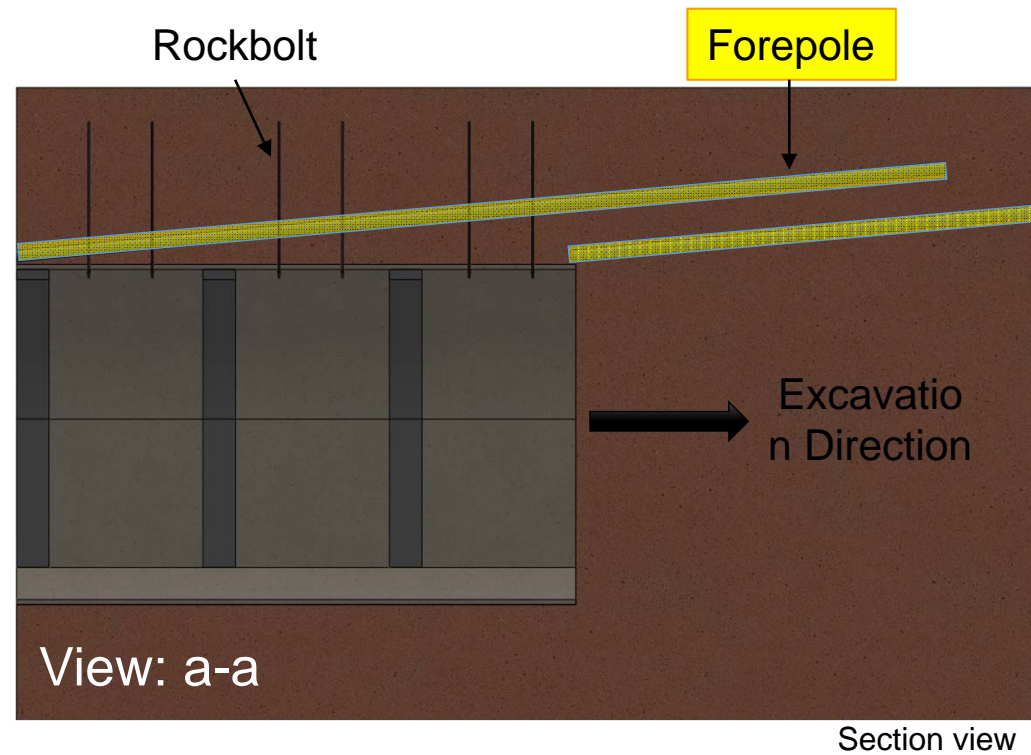
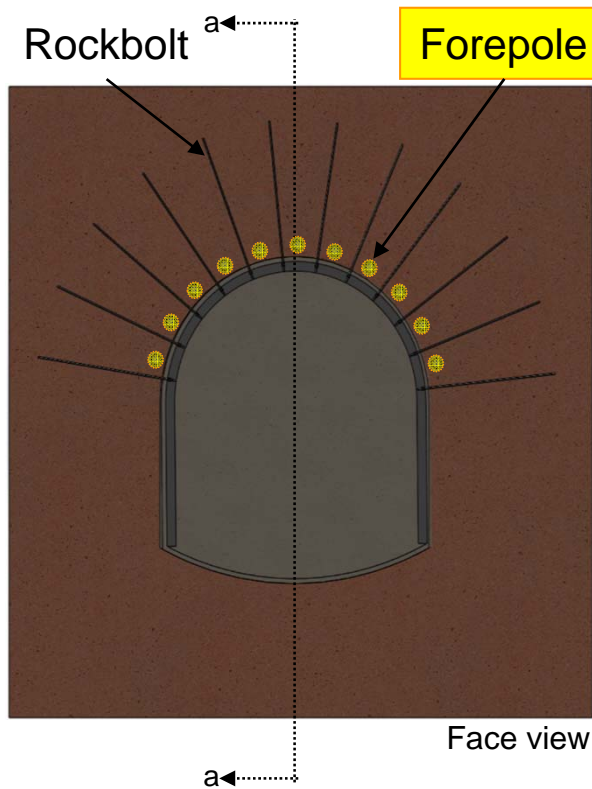
# Temporary Support Systems



(Modified after Solotrat, 2011)



# Temporary Tunnel Support Scheme

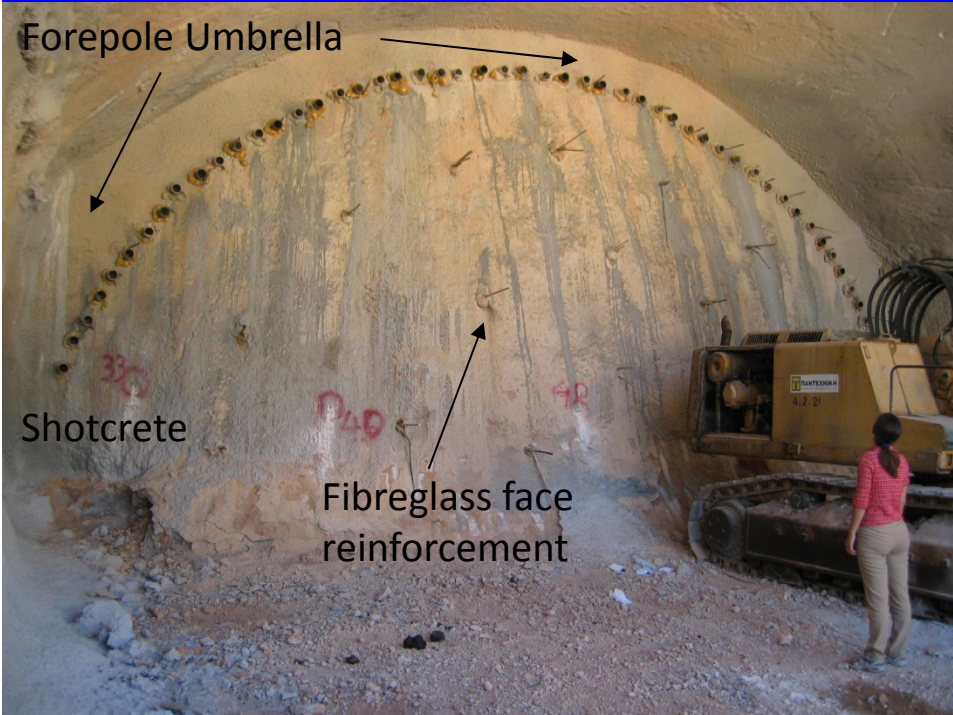




Forepoles



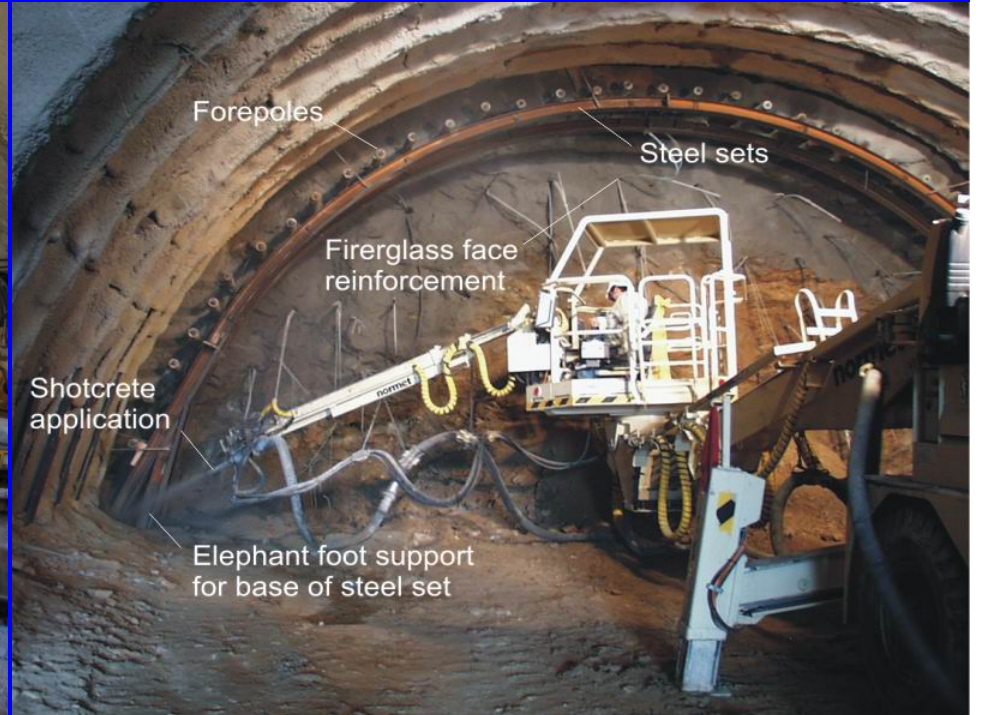
Forepoling machine



Forepole Umbrella

Shotcrete

Fibreglass face reinforcement



Forepoles

Steel sets

Fiberglass face reinforcement

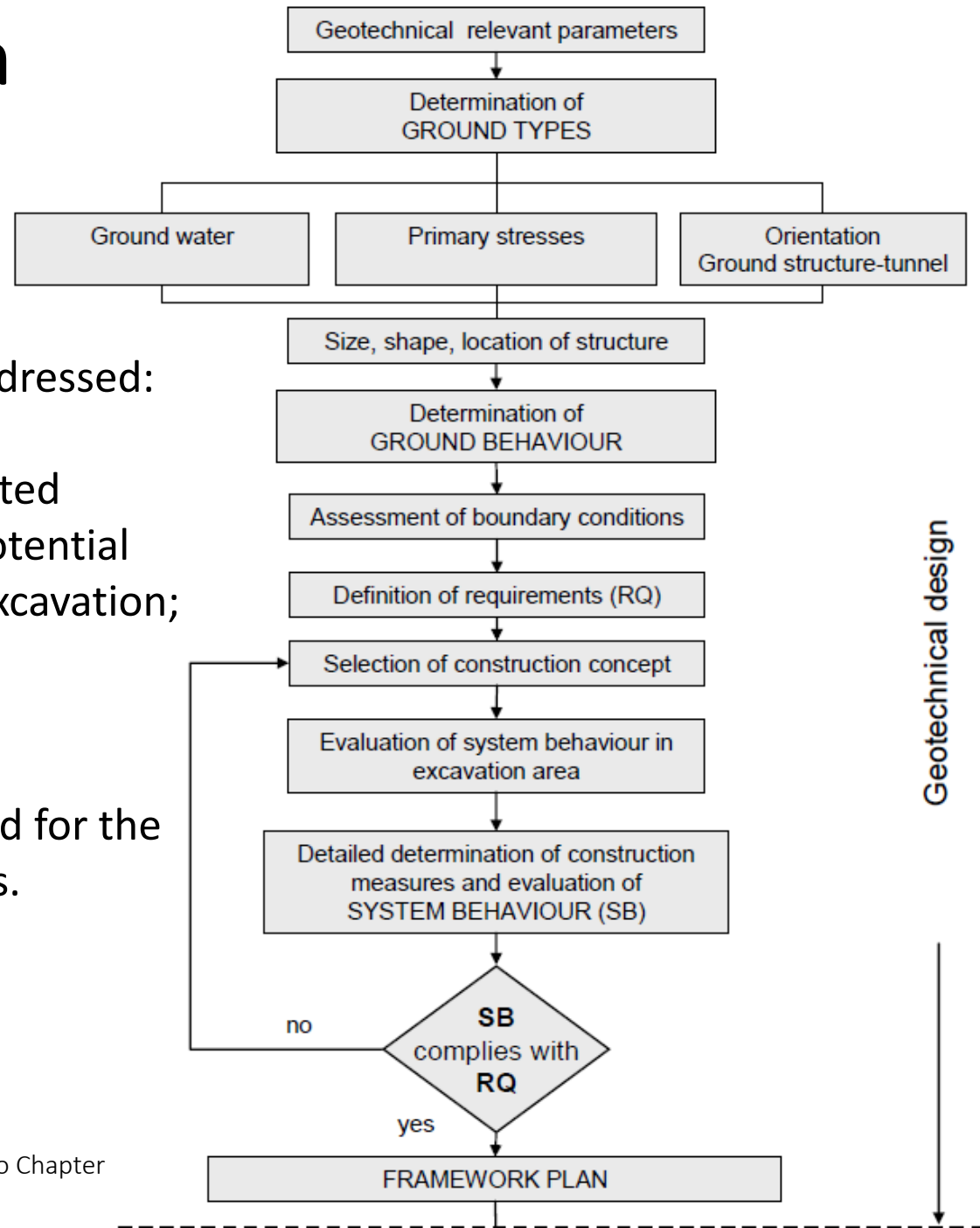
Shotcrete application

Elephant foot support for base of steel set

# Engineering Design

Two major aspects need to be addressed:

- Realistic estimate of the expected ground conditions and their potential behaviours as a result of the excavation; and,
- Design and economic and safe excavation and support method for the determined ground behaviours.



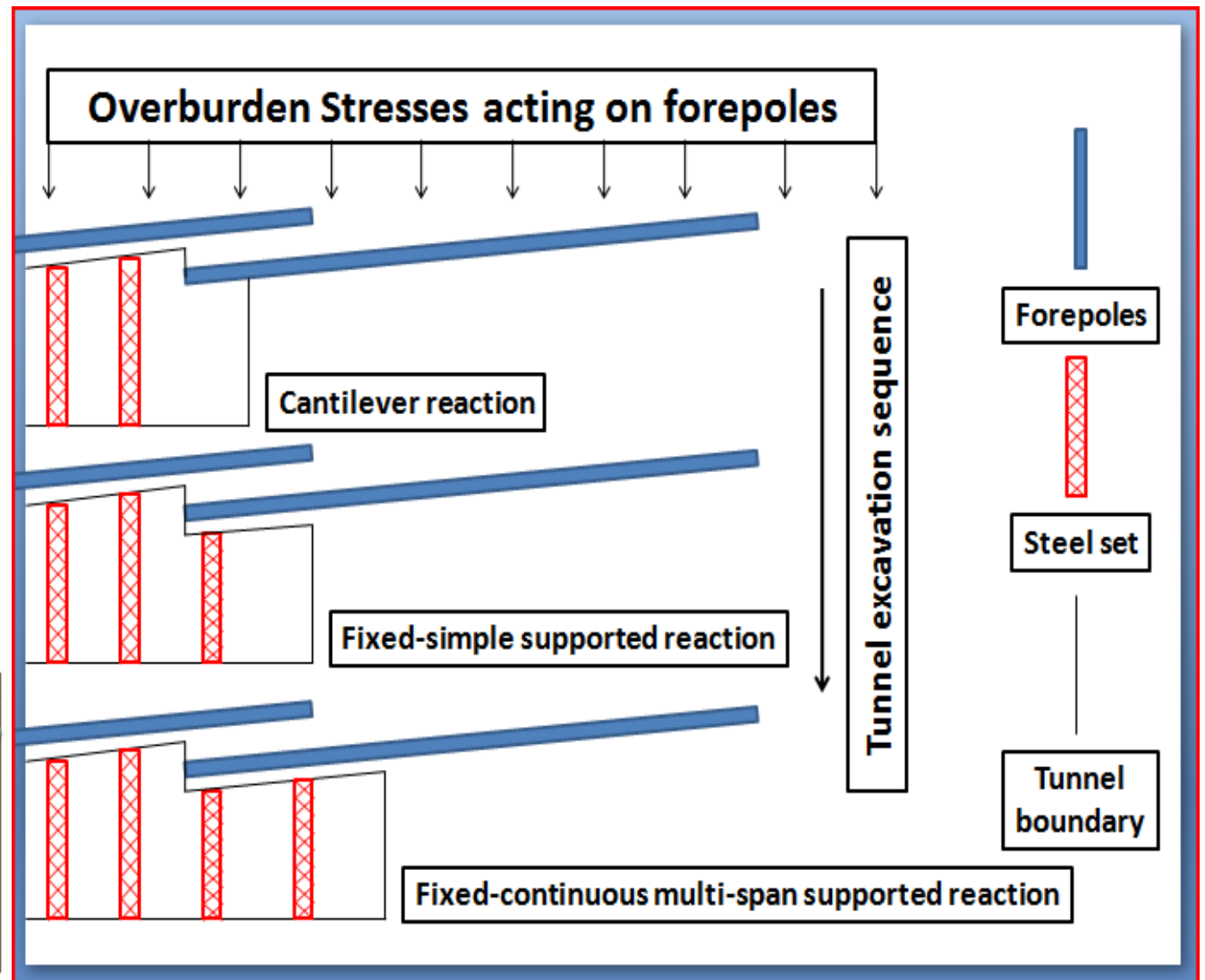
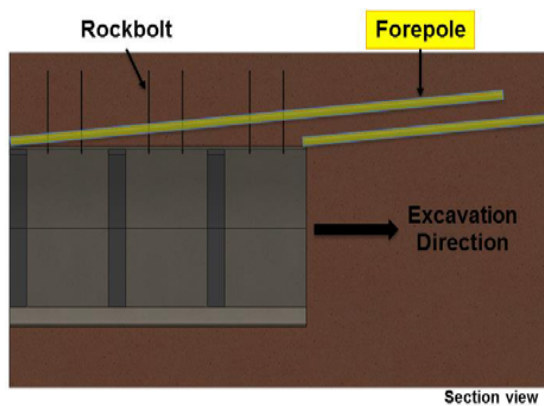
# Design of Umbrella Arches (UA)

The design of UAs is complex due to:

- 3D geometry of tunnel face;
- high number of parameters involved;
- Influence of tunnel face – stability;
- 3D analysis to take into account face position;
- Overlapping of the successive arches;
- Connection between support elements and steel sets; and,
- Stiffness and quality of steel sets.



# Mechanisms



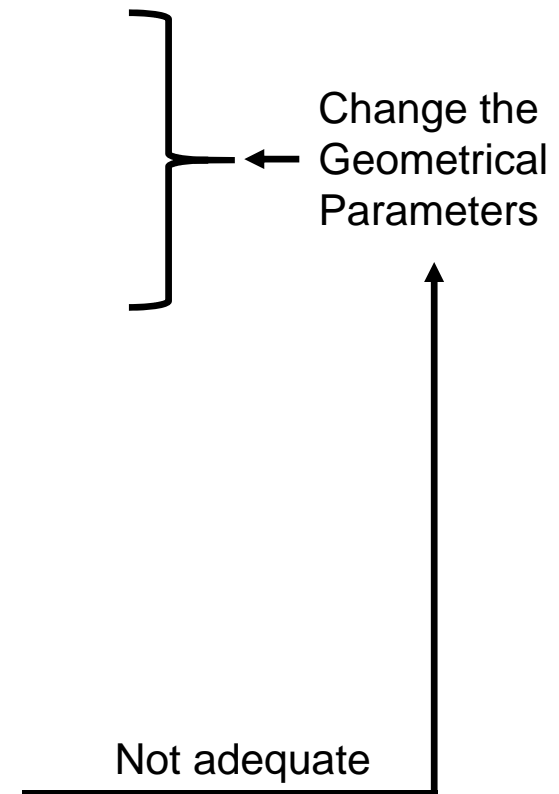
(Jeff Oke, Grad Student 2013)



# Analytical Design Approaches

## Design Procedure

1. Choice of an initial forepole spacing
2. Choice of an initial diameter and thickness of forepole
3. Choice of an initial steel set spacing
4. Choice of forepole (steel pipe)
5. Evaluation of Design Length
6. Choice of Structural Schema
7. Computation of Bending Moments
8. Computation of Stresses in forepole
9. Compare acting stress and steel yield stress
10. Definition of the forepole length (machines available)
11. Definition of excavation length
12. Design of the supports and steel arch foundations



Peila, 2013



# Geo-Mechanical Understanding

*“ The rapid increase in use [of umbrella arch systems] has not been followed by an increased understanding of the interactions between the support system and the surrounding rock mass”*

“ Currently during tunnel design, as well as on site, parameters for the [umbrella arch] system... are fixed by experience or an empirical approach”

Volkmann (2003)





# Laboratory Testing



Embedded rebar specimen



Forepole Specimen

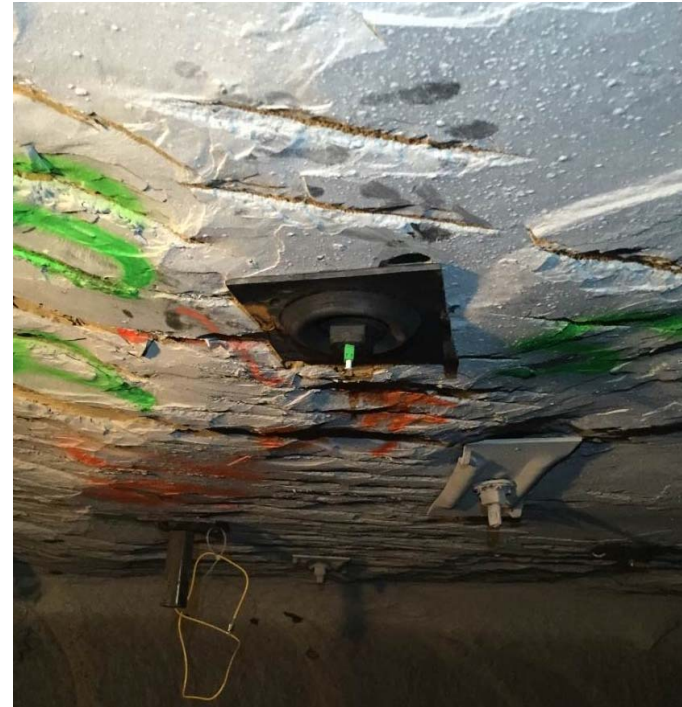
## Specimens Subjected to:

- Bending
- Axial loading
- Shearing



# Distributed Optical Sensing (DOS)

- 1.25mm spatial resolution (continuous strain profile)
- Operational accuracy is within  $\pm 10\mu\epsilon$
- Is based on low-cost, standard telecom fiber
- One transducer for thousands of measurement points



DOS instrumented bolt installed in the roof of a coal mine



# Continual Strain Monitoring Testing – Rock Bolt



- Developed a prototype



Diametrically opposed grooves  
along the length of a Rebar Bolt

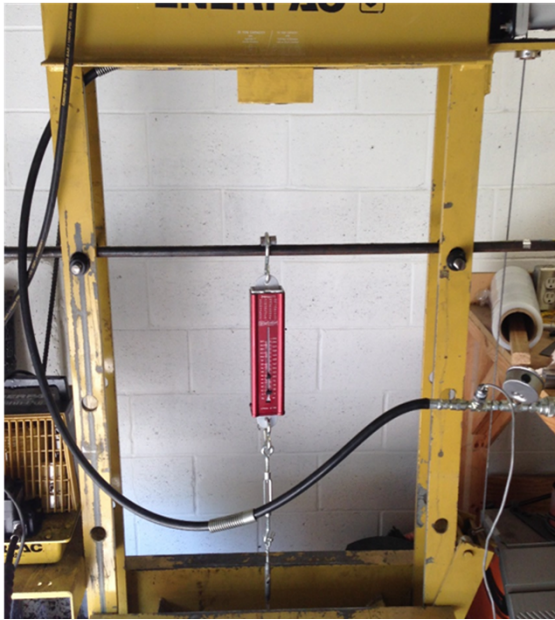
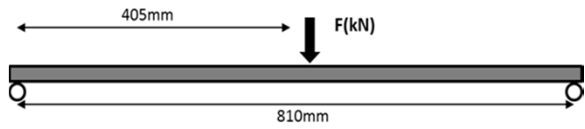


Run fiber- optic instrumentation  
along the grooves in Rebar

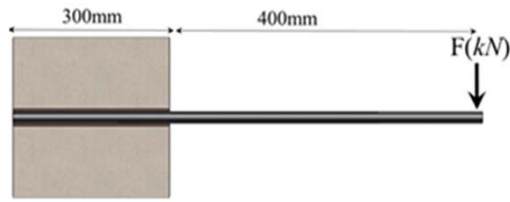


- Tests conducted on ASTM A53 steel pipe:
  - 114mm OD, 6.02mm wall
  - 21.3mm OD, 2.77mm wall
- Optical instrumentation embedded into 2mm machined groove, as well as surface mounted
- Multiple epoxy resins and adhesives experimented with to bond instrumentation

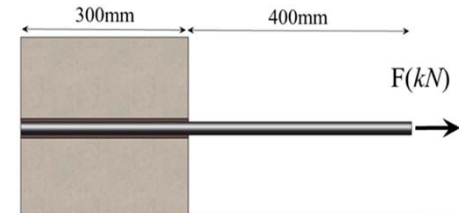




Three Point Bending



Cantilever Bending

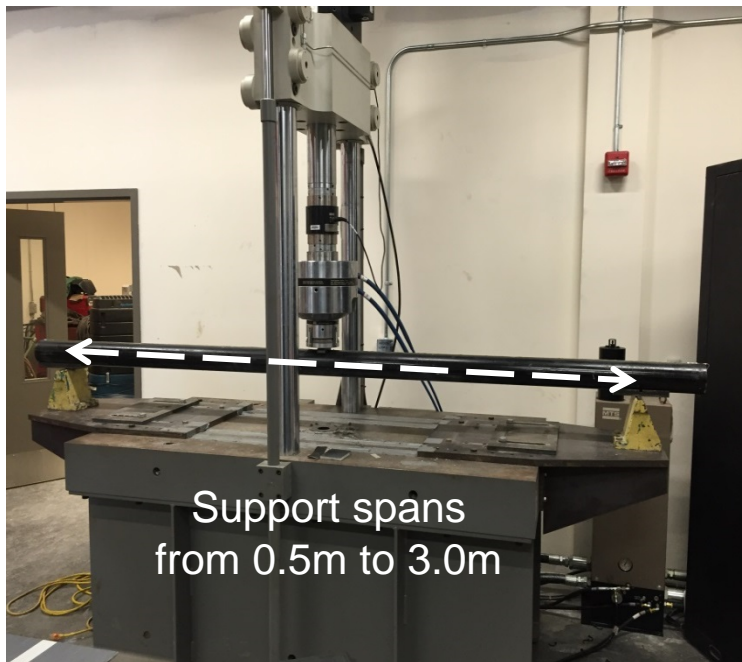


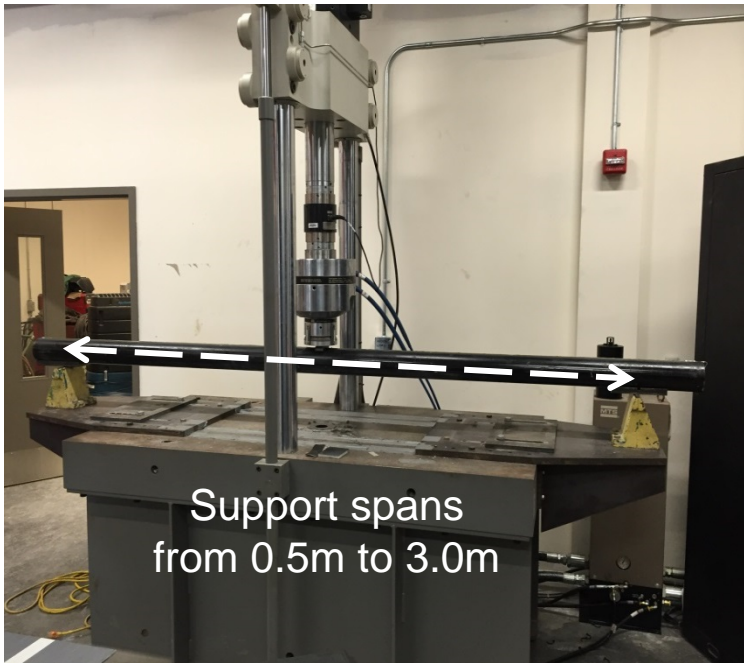
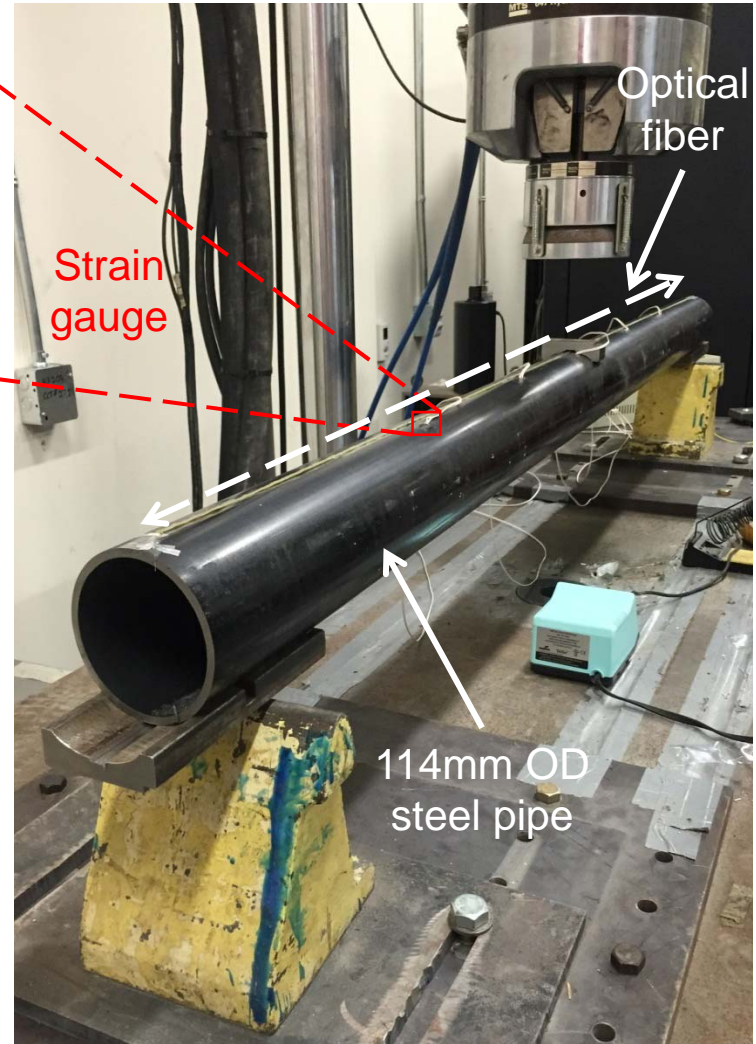
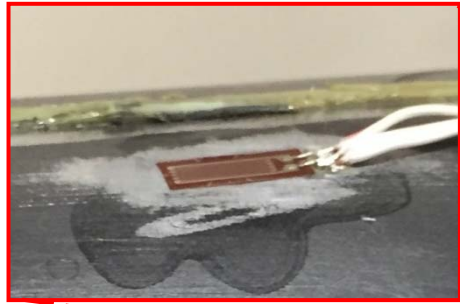
Axial Loading

(Brad Forbes, Grad Student 2014)

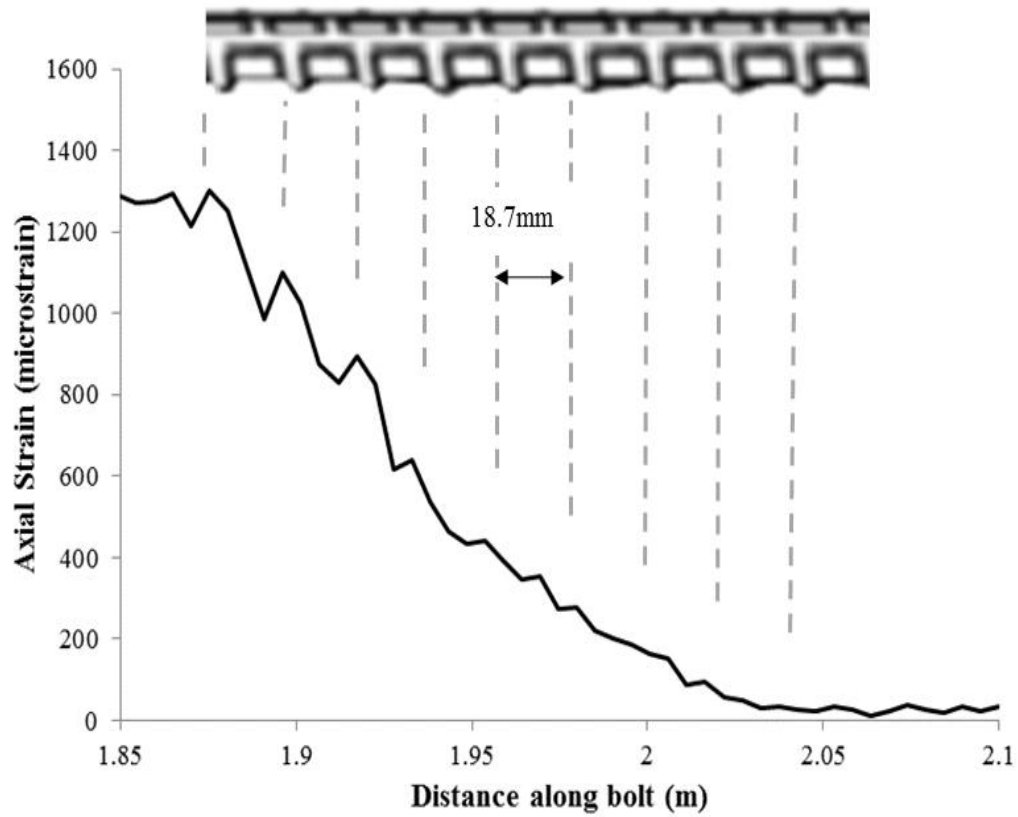


4 inch schedule 40 ASTM A53  
Grade B steel pipe



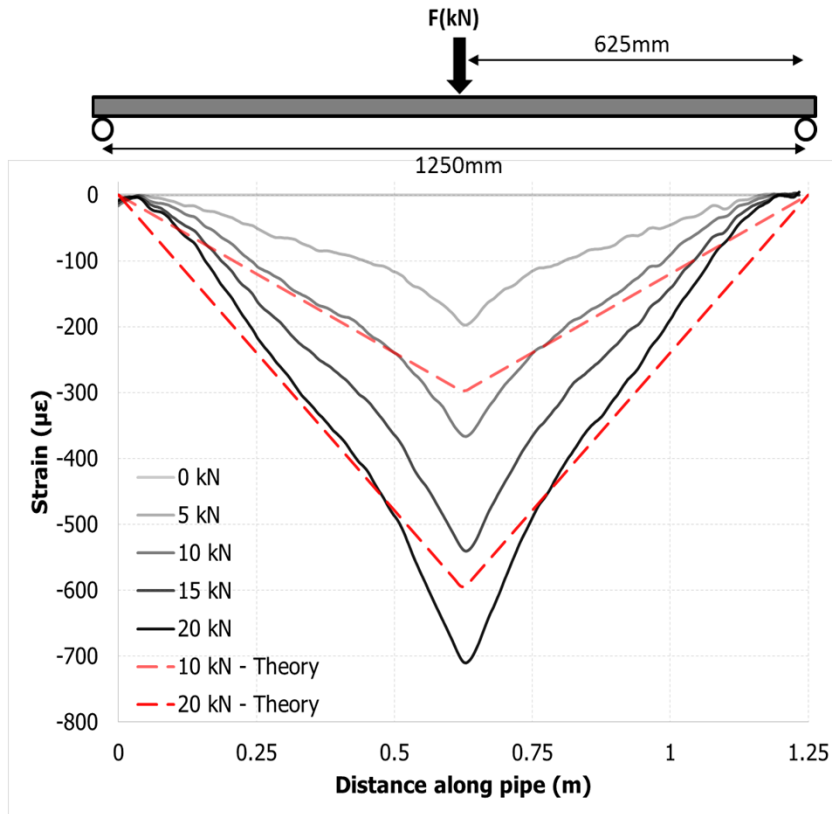


# Pullout-Test

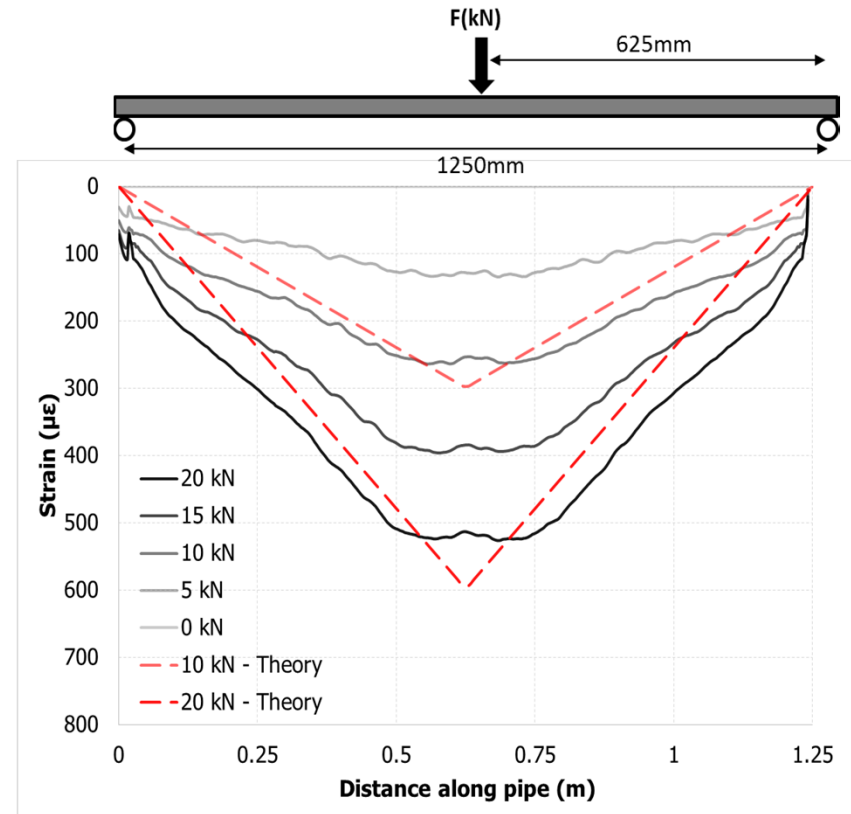




# Bending-Test



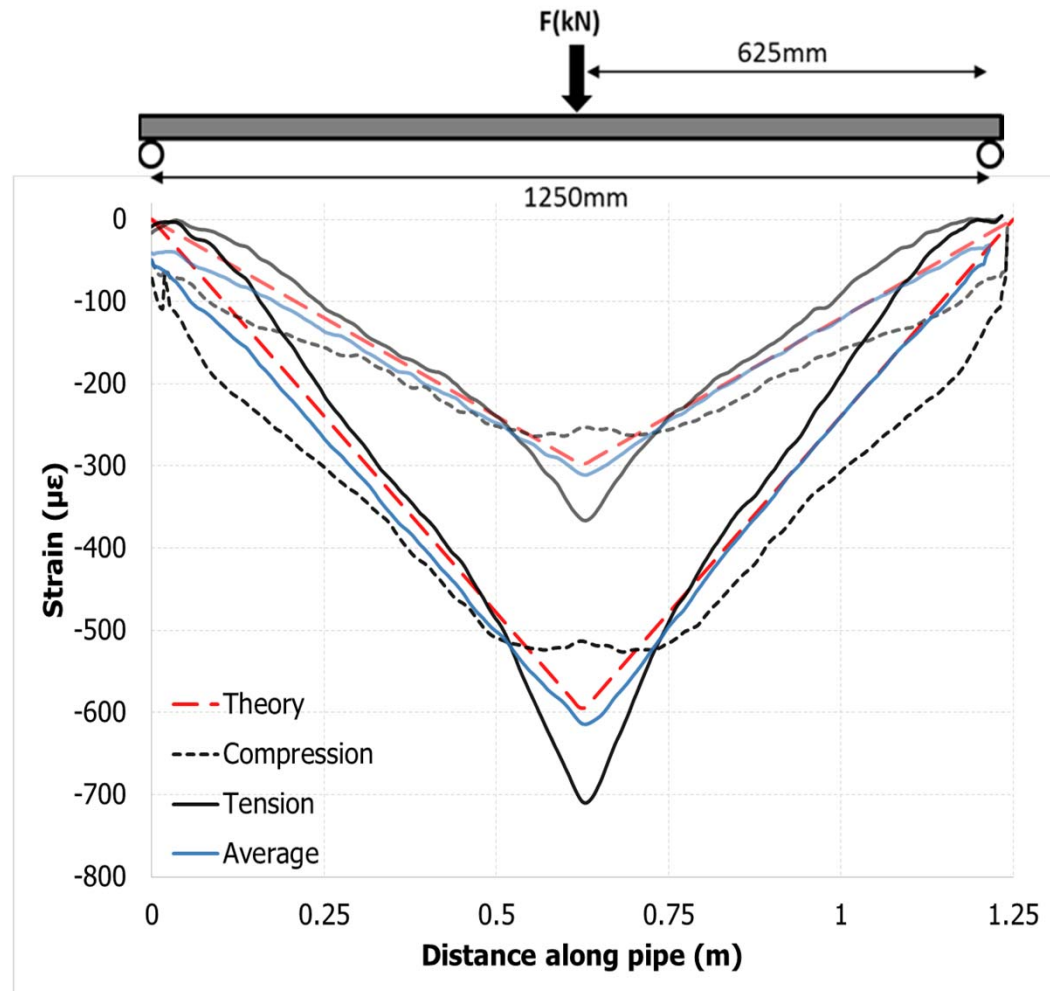
top section of the pipe in compression



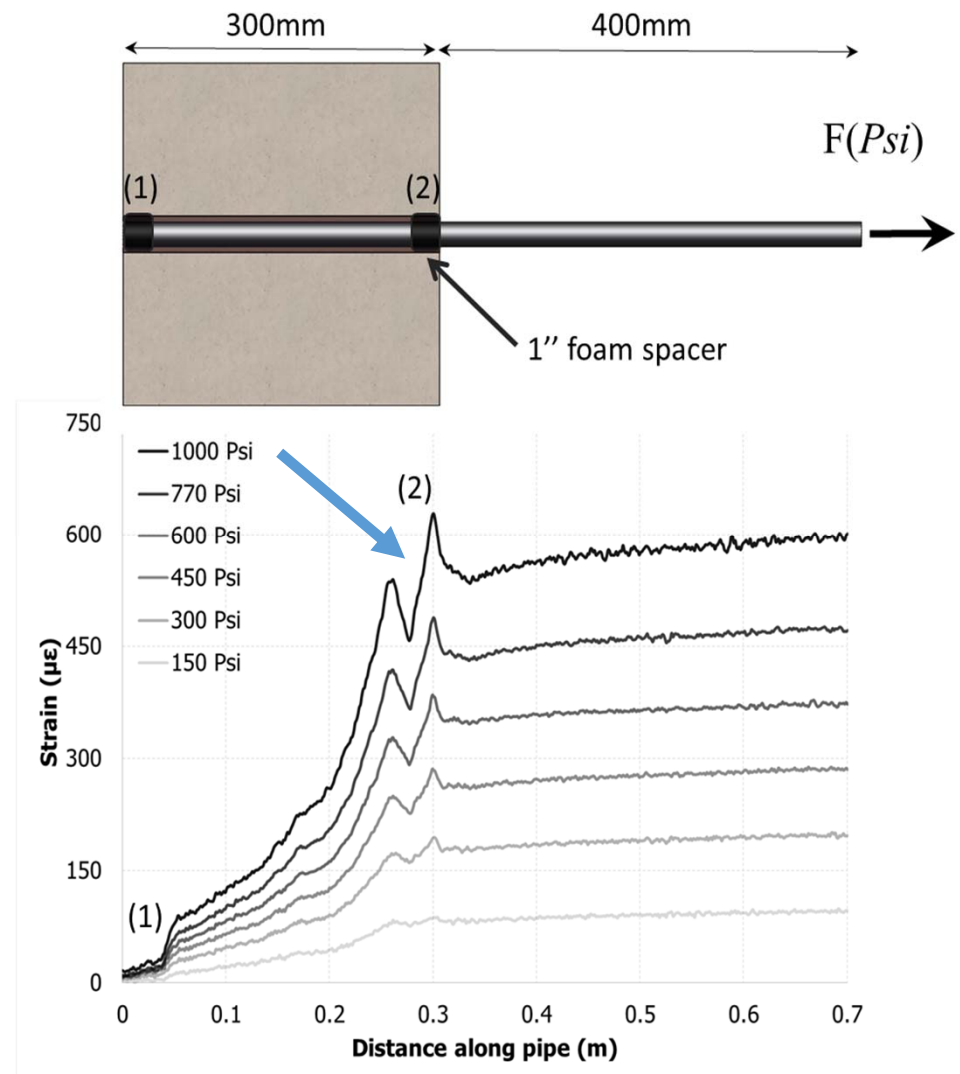
bottom section of the pipe in tension

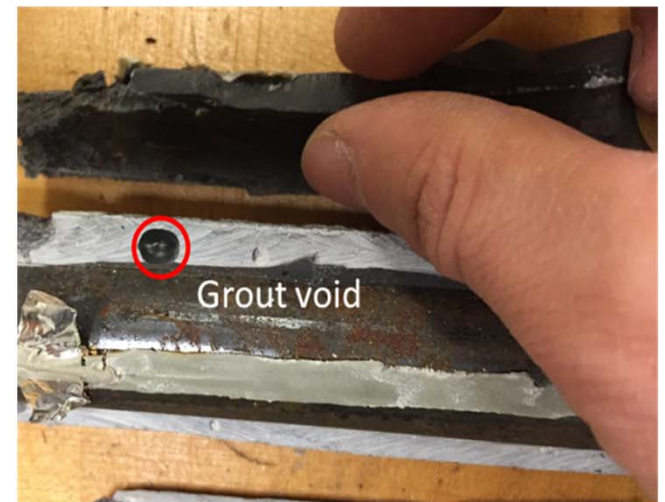
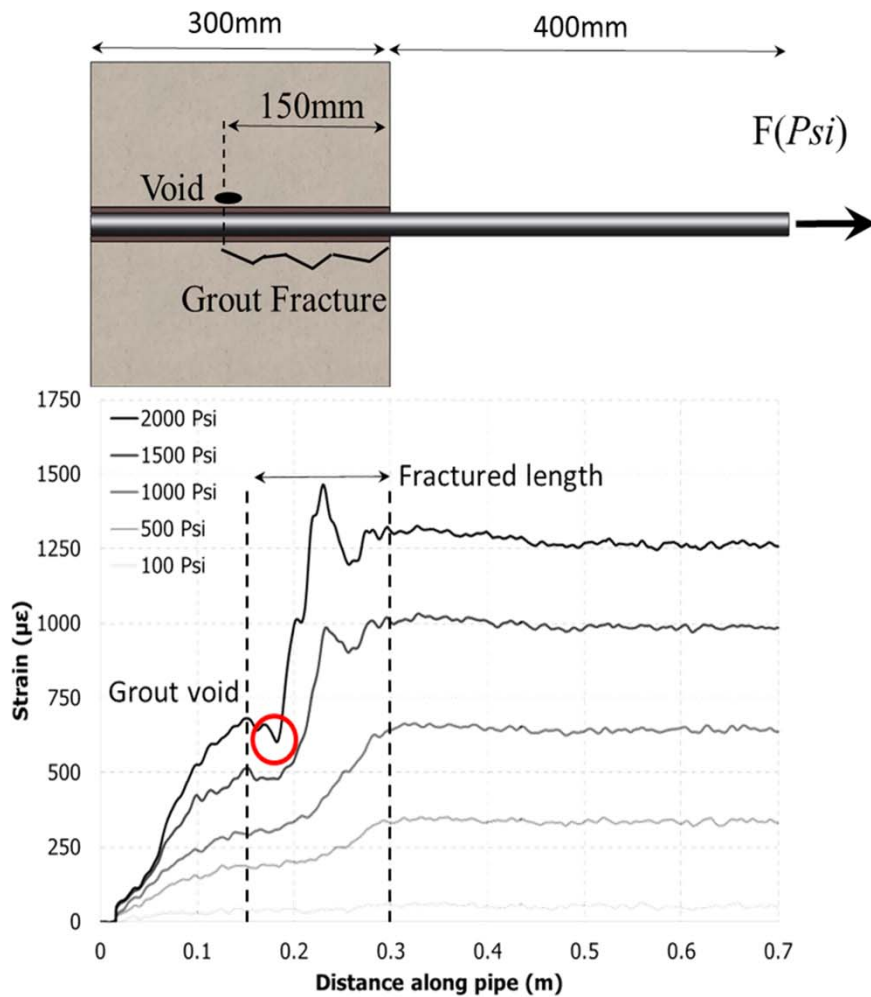


- Absolute average of the top and bottom section of the pipe under bending
- Yields an approximately linear response



- The strain profile shows inconsistencies at the end of the grouted section.
- These correspond to 1 inch long foam spacers used to center the steel pipe while grouting

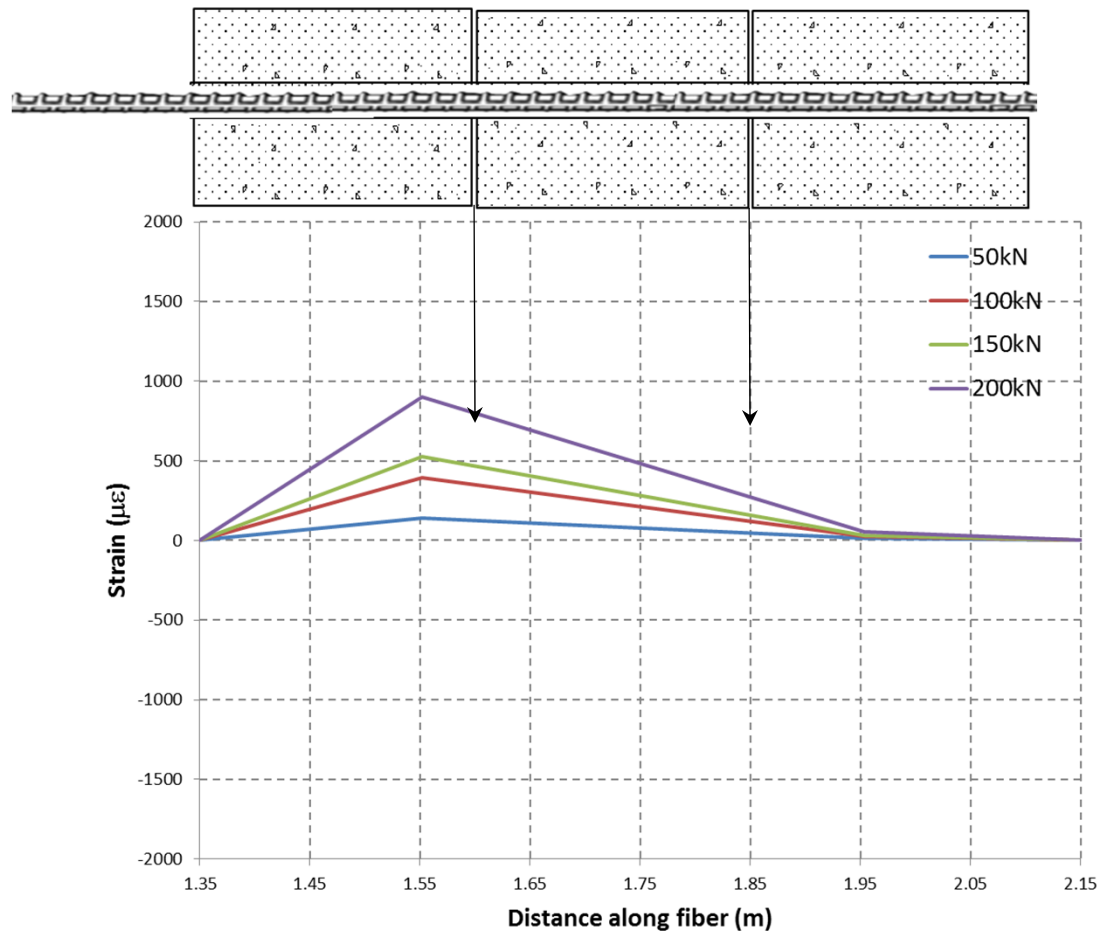




Shows the potential for the optical technique to be used to detect minor features



# Double Shear Configuration



## Spatial Resolution

40cm

25cm

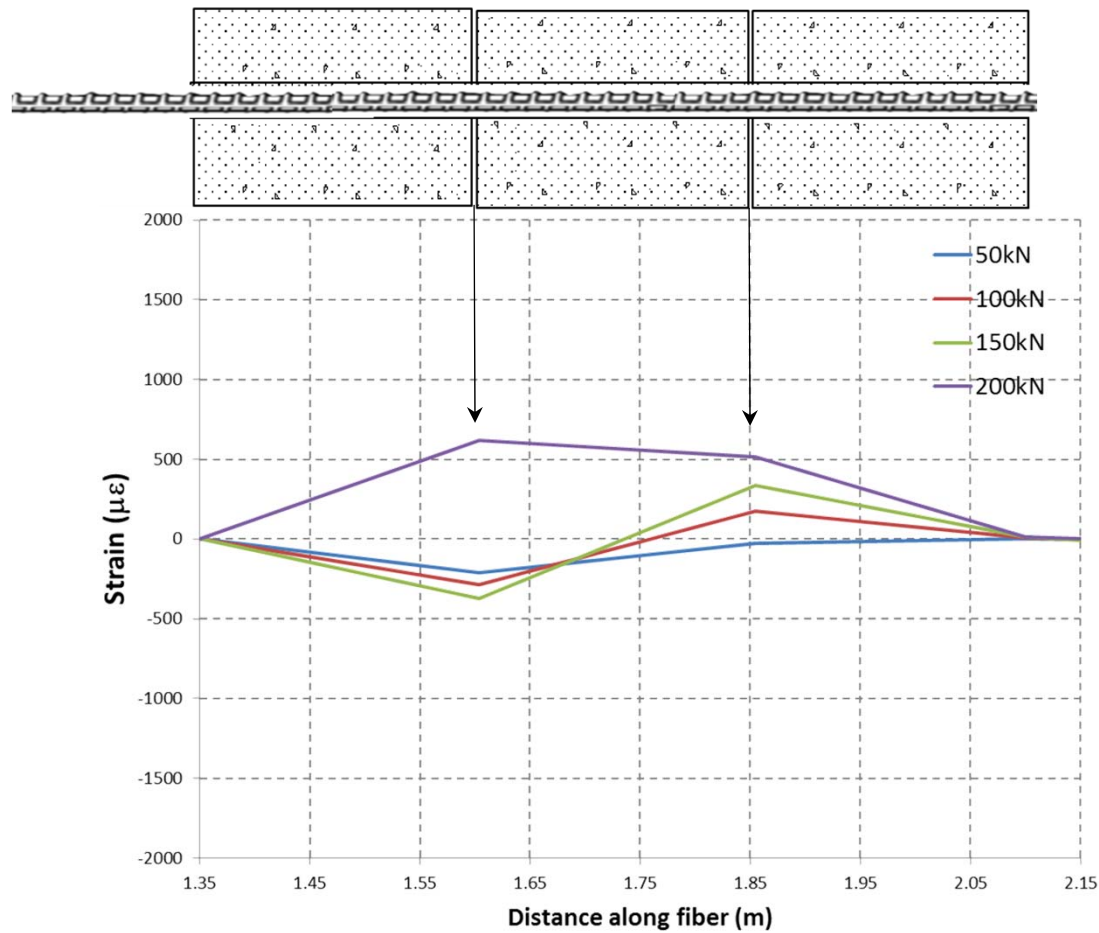
10cm

1.25mm

Experimental Plot – Top section of rebar



# Double Shear Configuration



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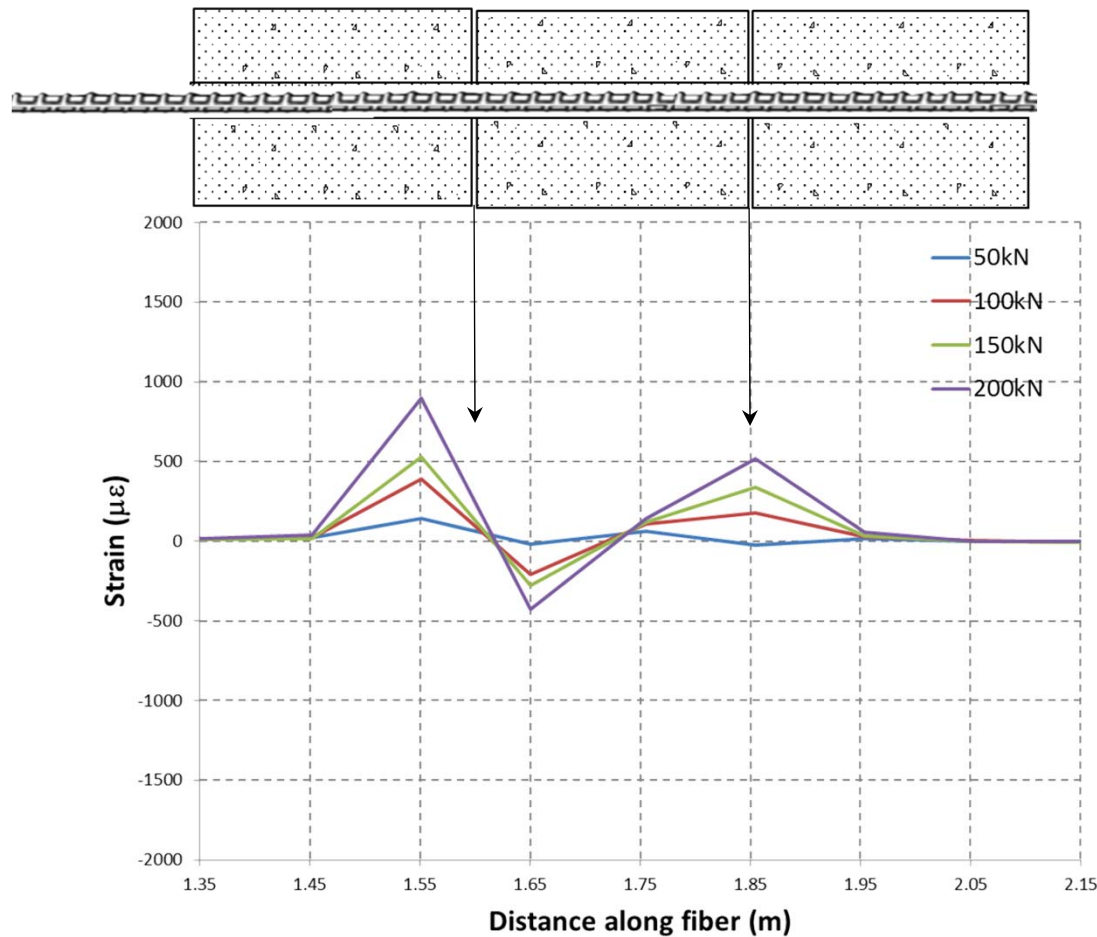
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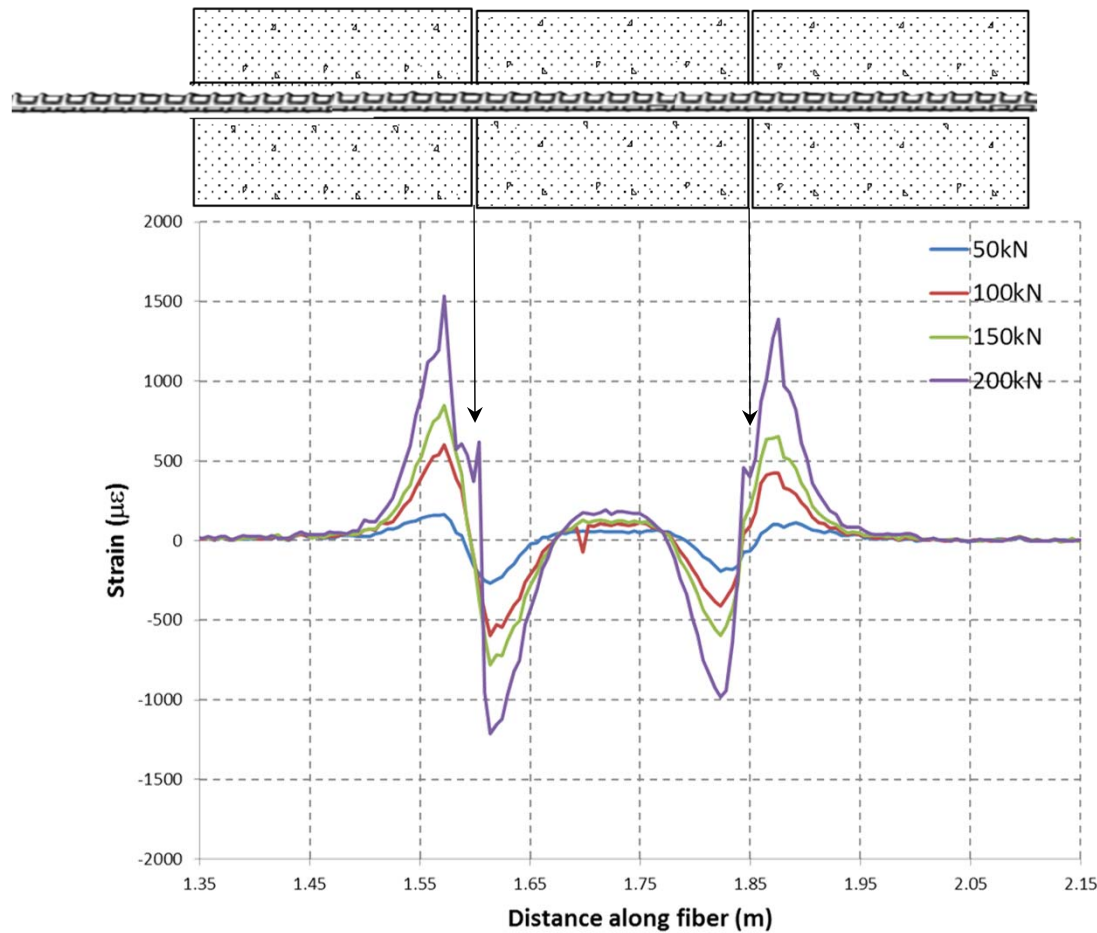
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Experimental Plot – Top section of rebar



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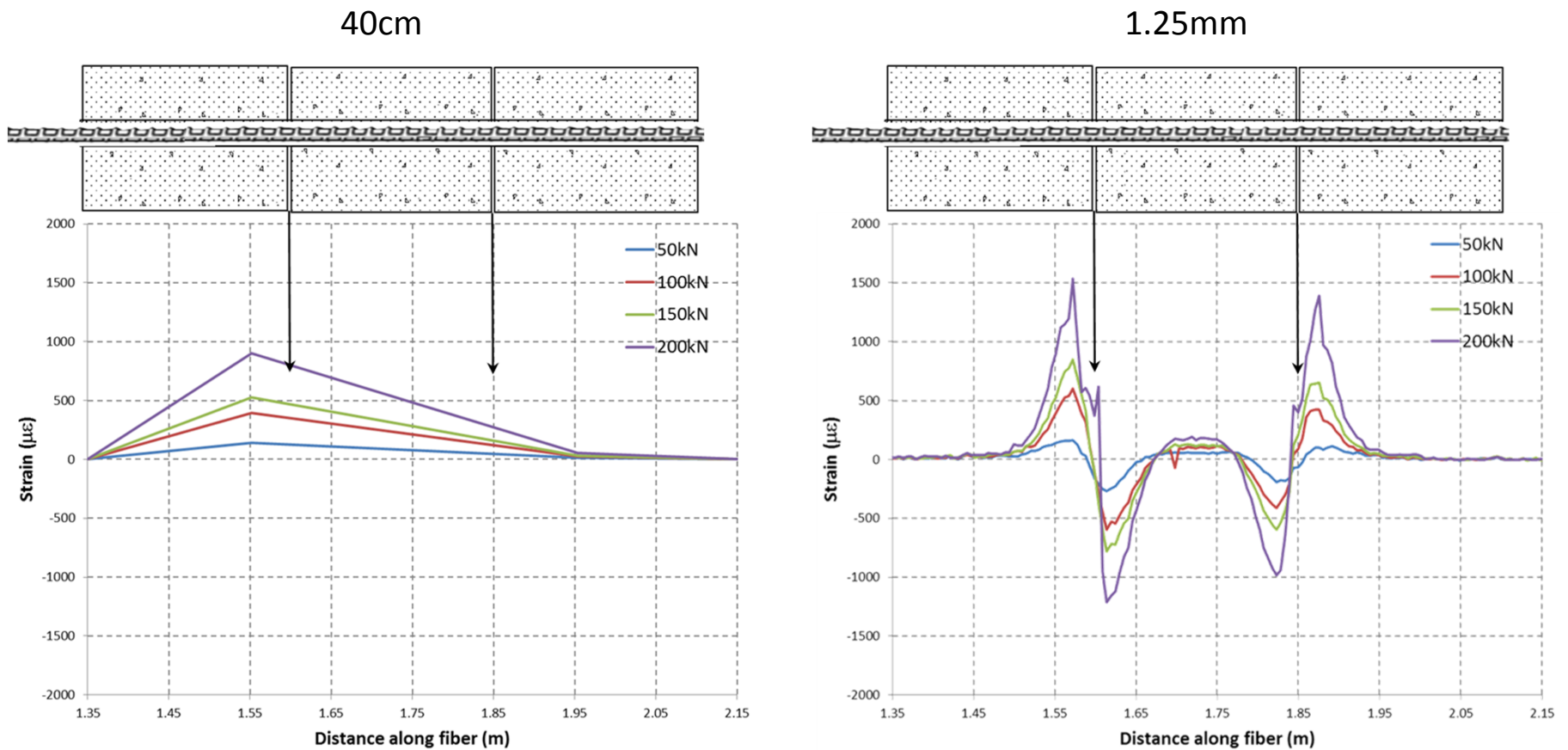
1.25mm

Experimental Plot – Top section of rebar





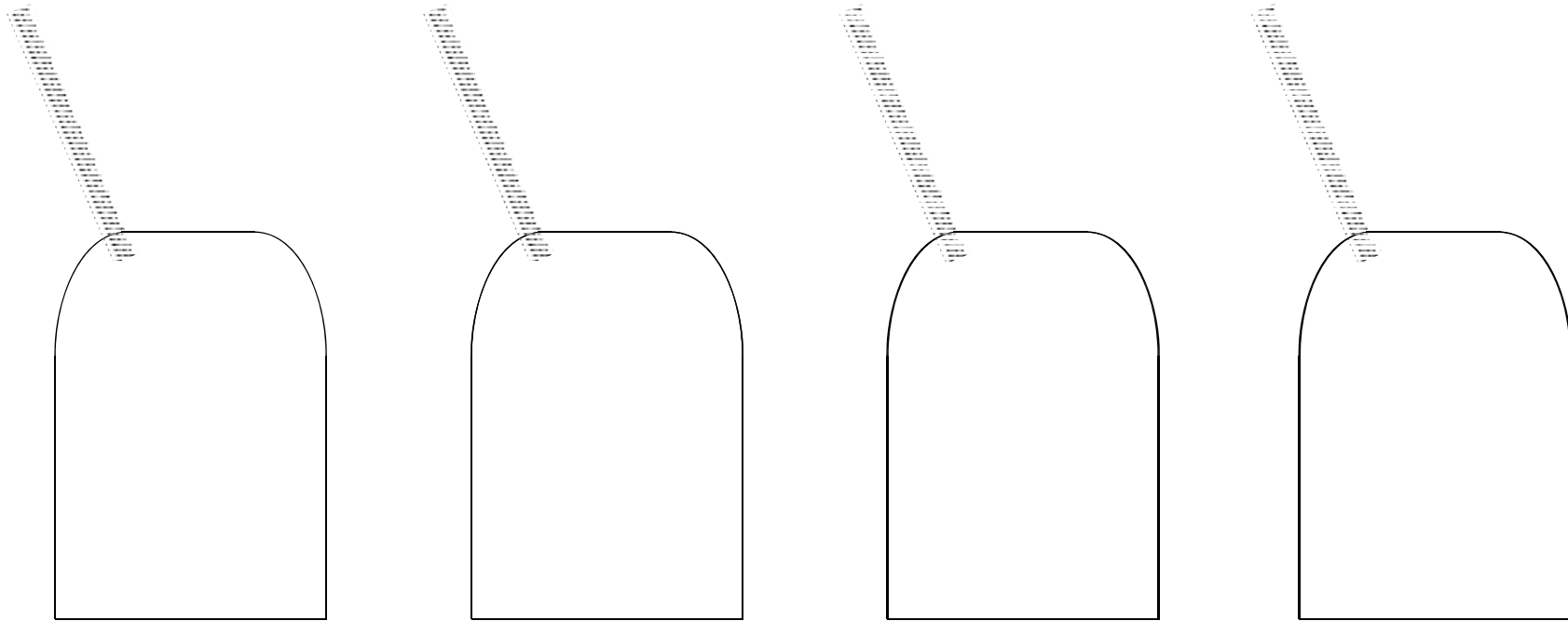
# Double Shear Configuration



Experimental Plot – Top section of rebar



# Is Continuous Monitoring Necessary?



(1)

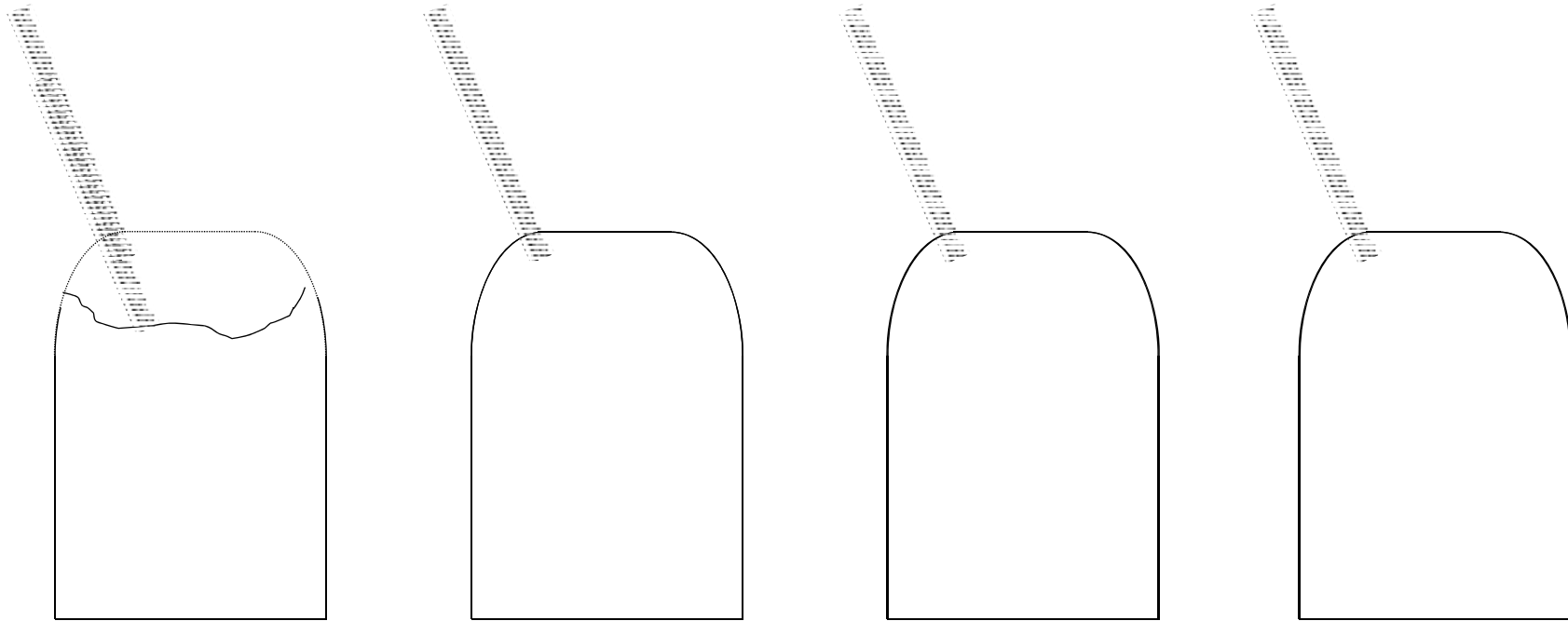
(2)

(3)

(4)



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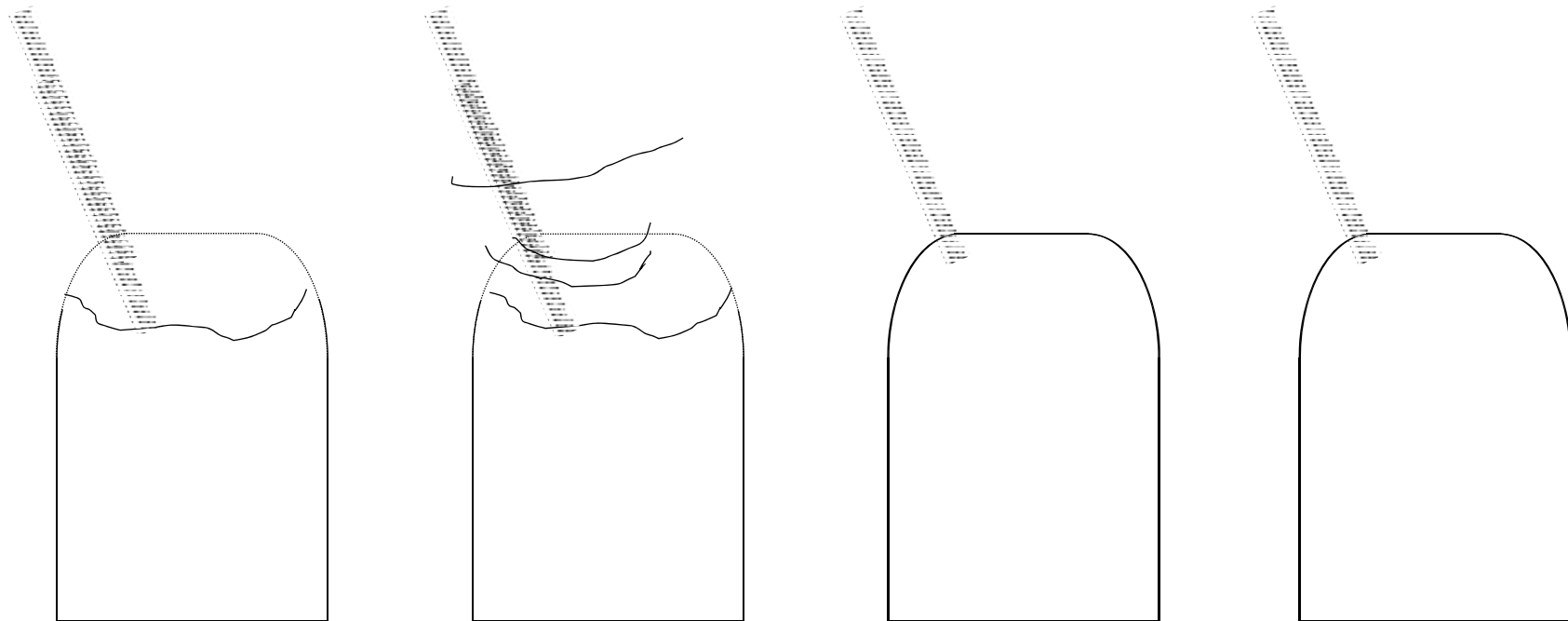
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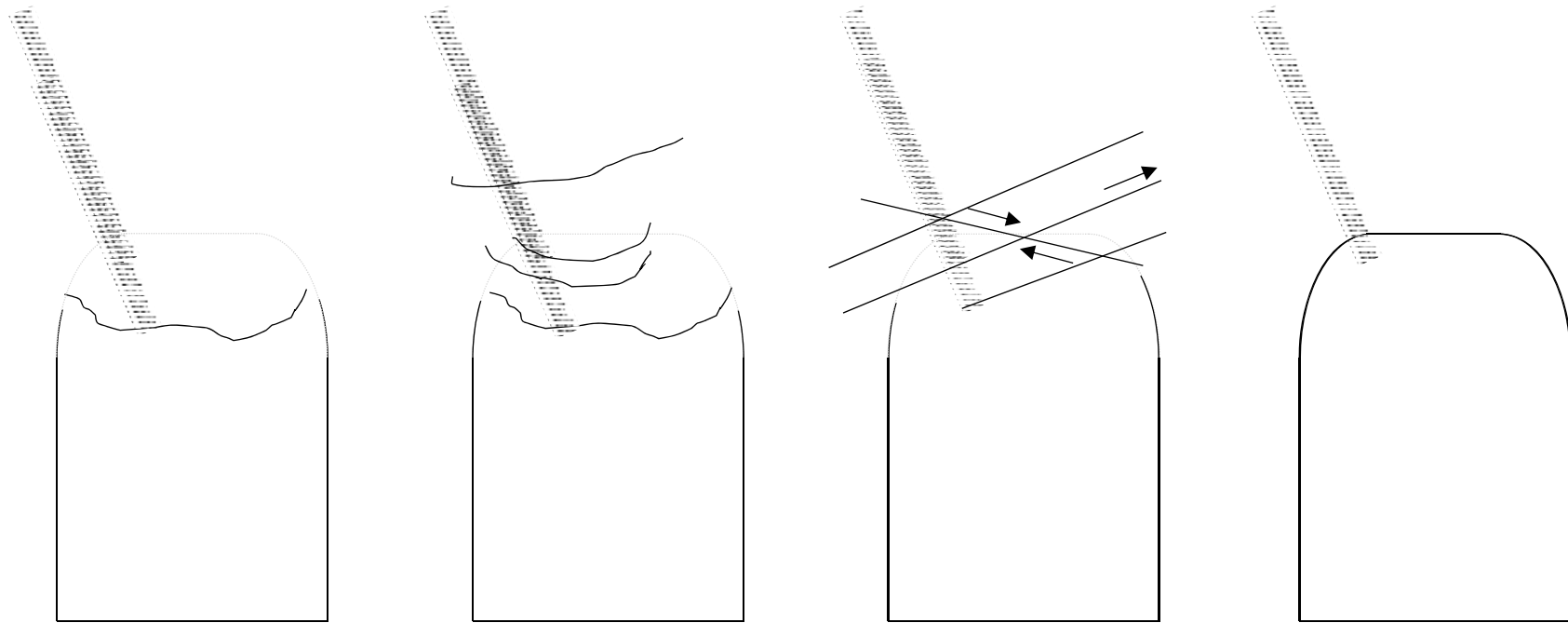
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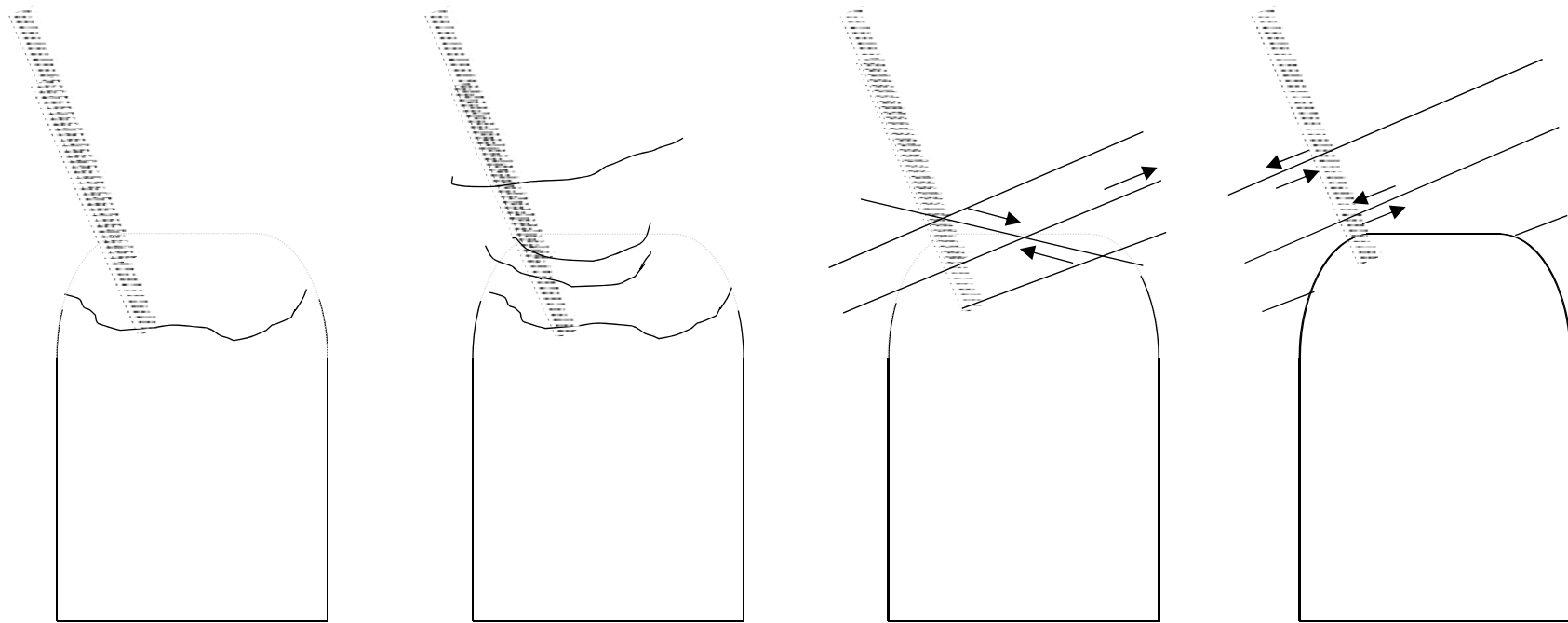
(2)

(3)

(4)



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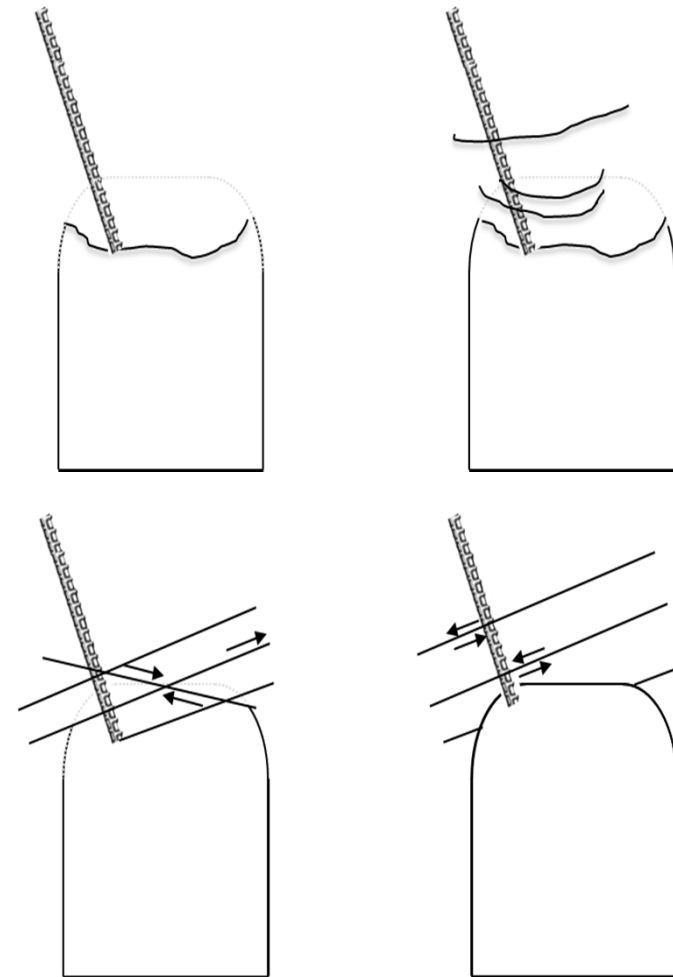
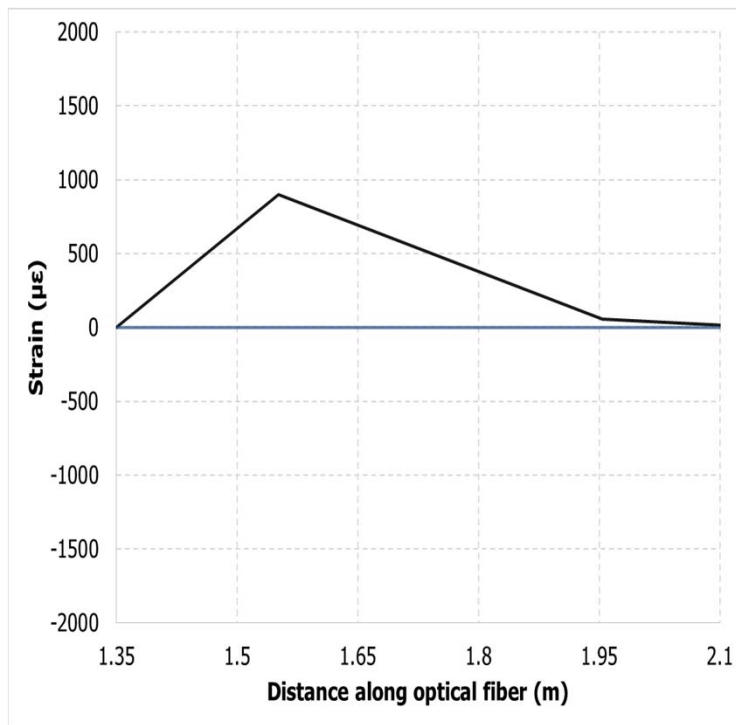
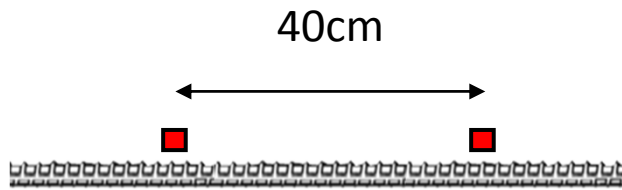
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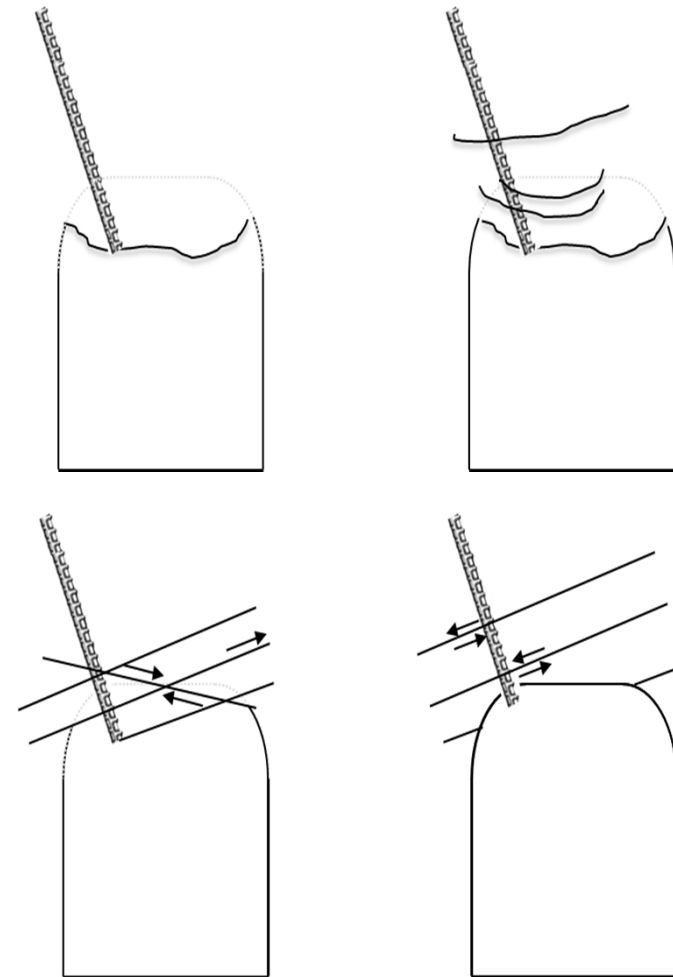
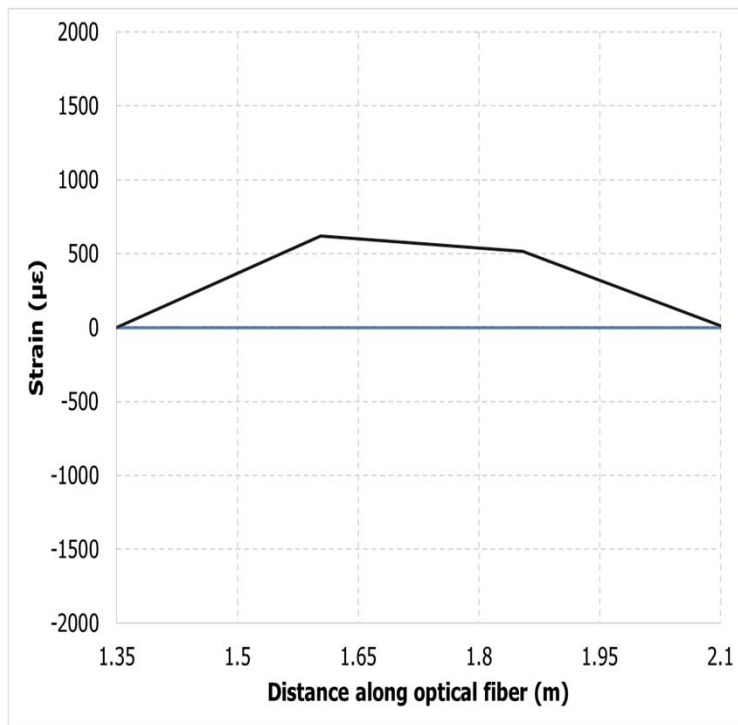
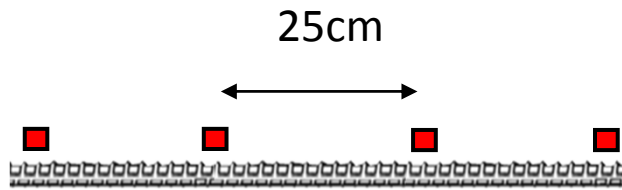
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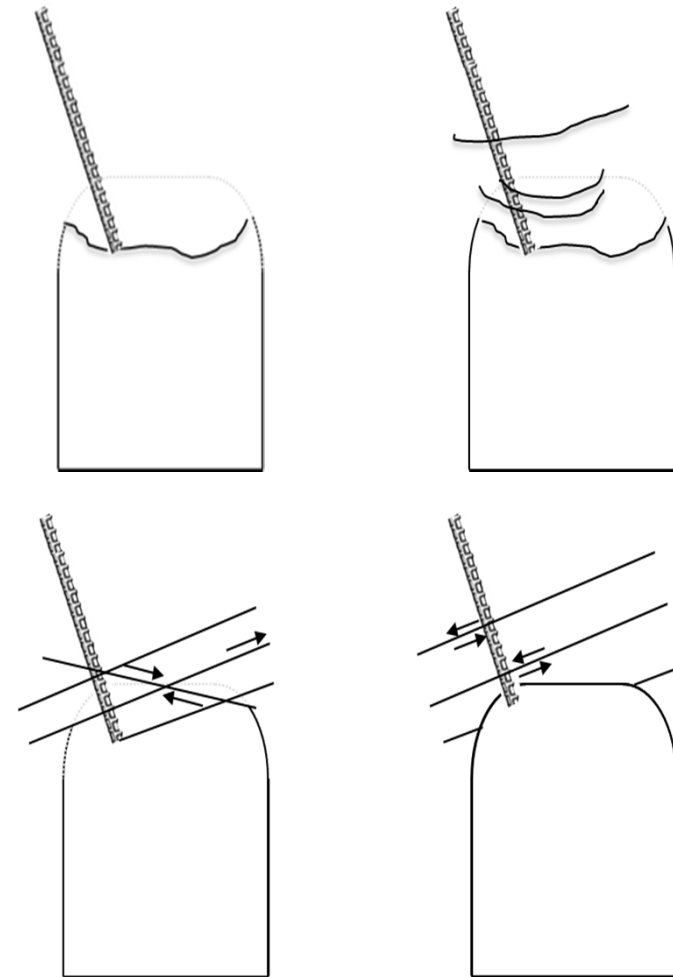
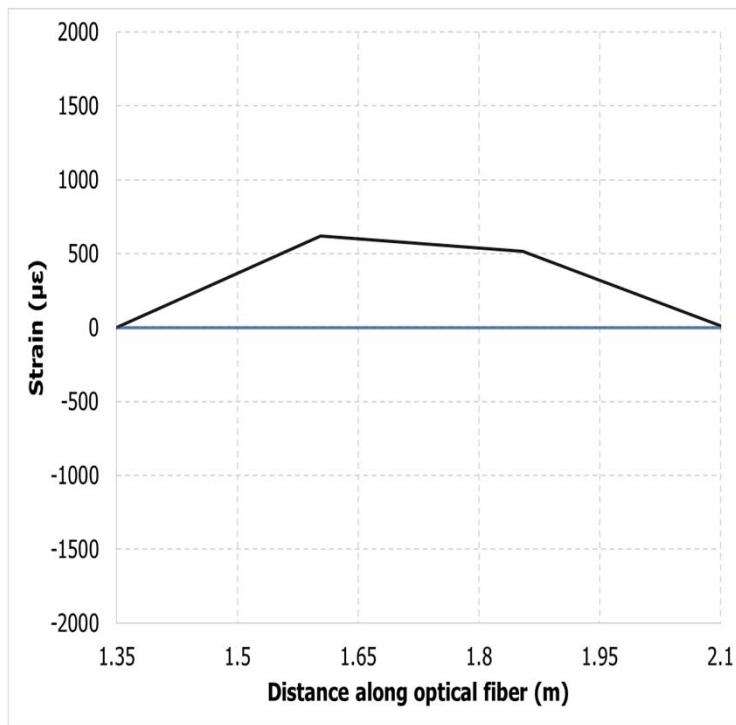
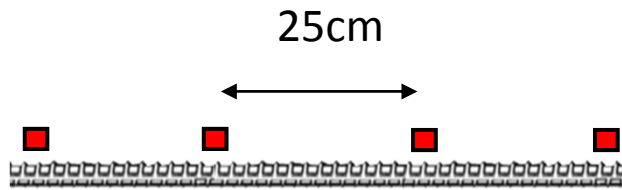


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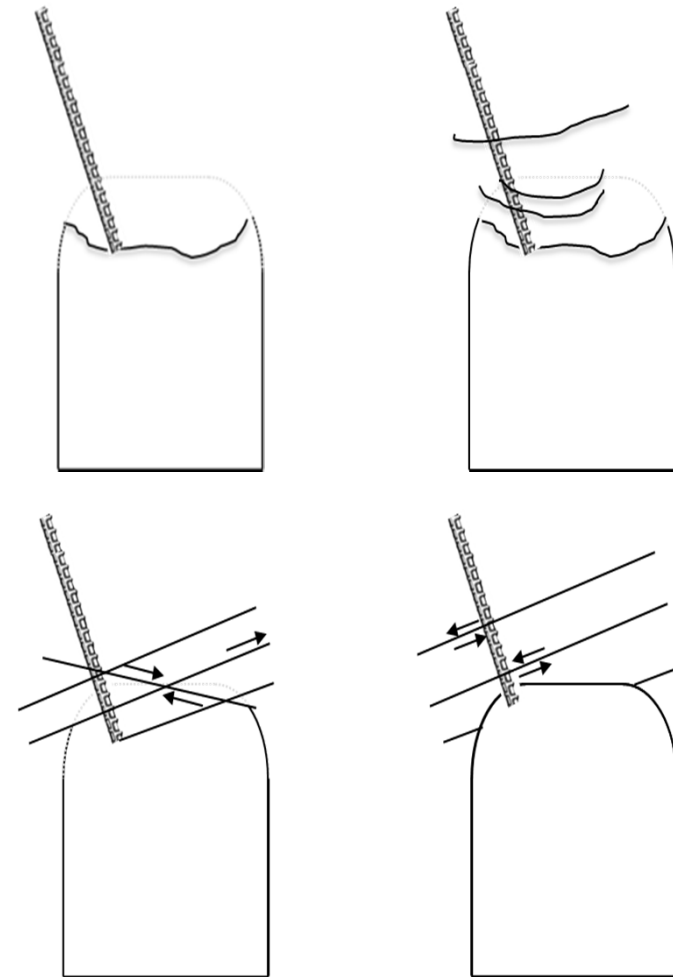
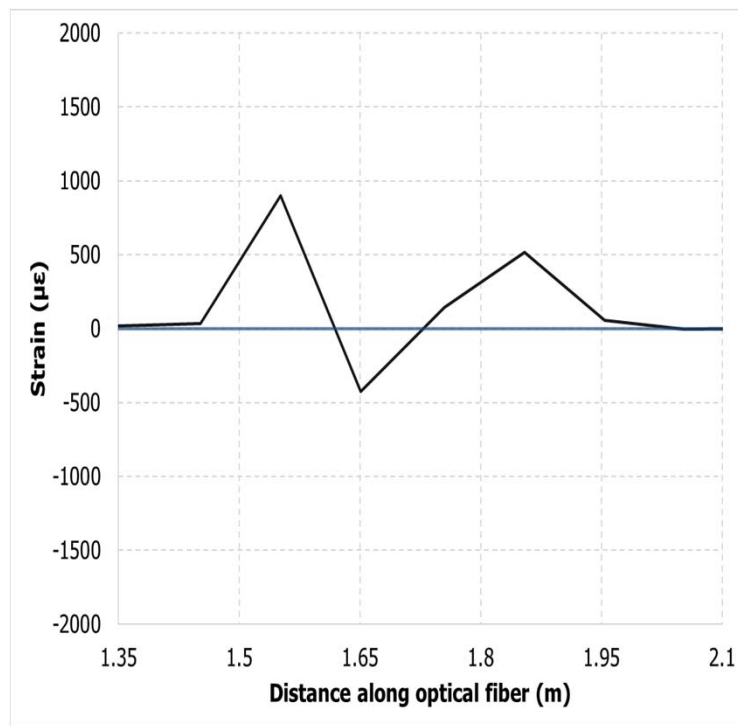
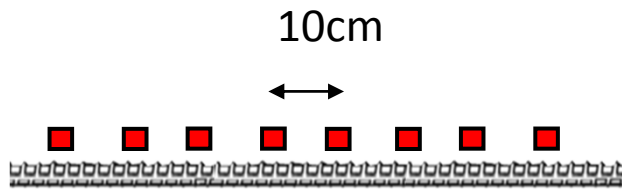




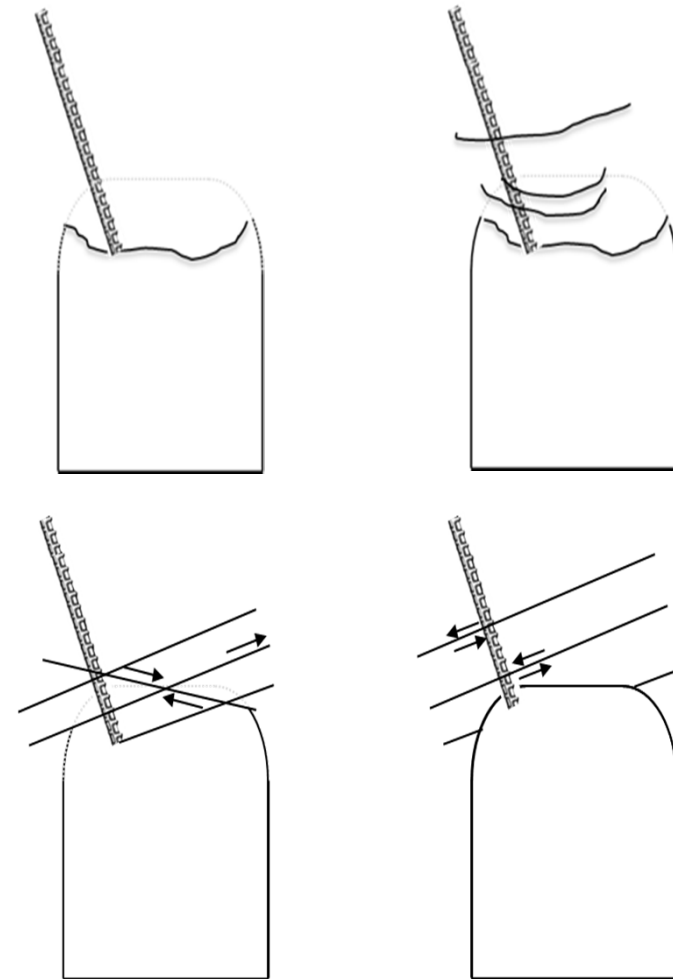
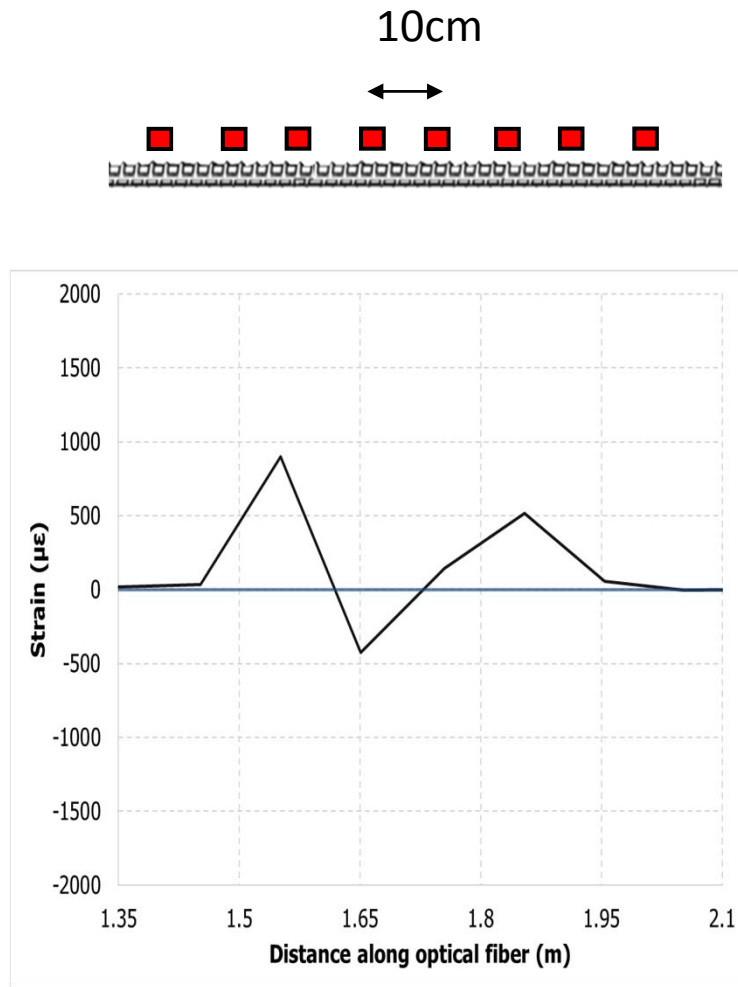
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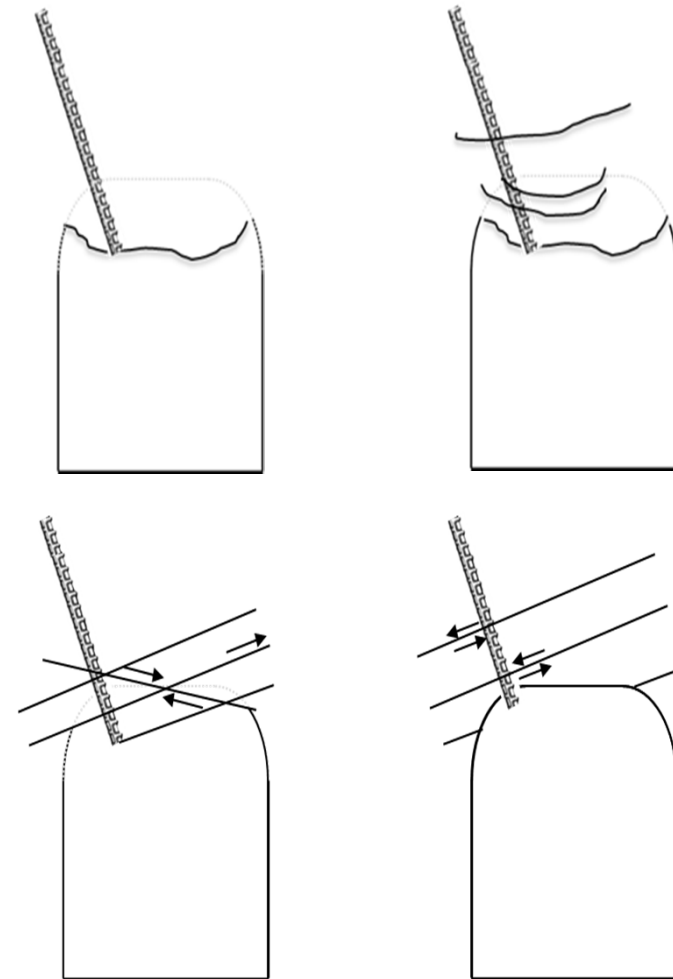
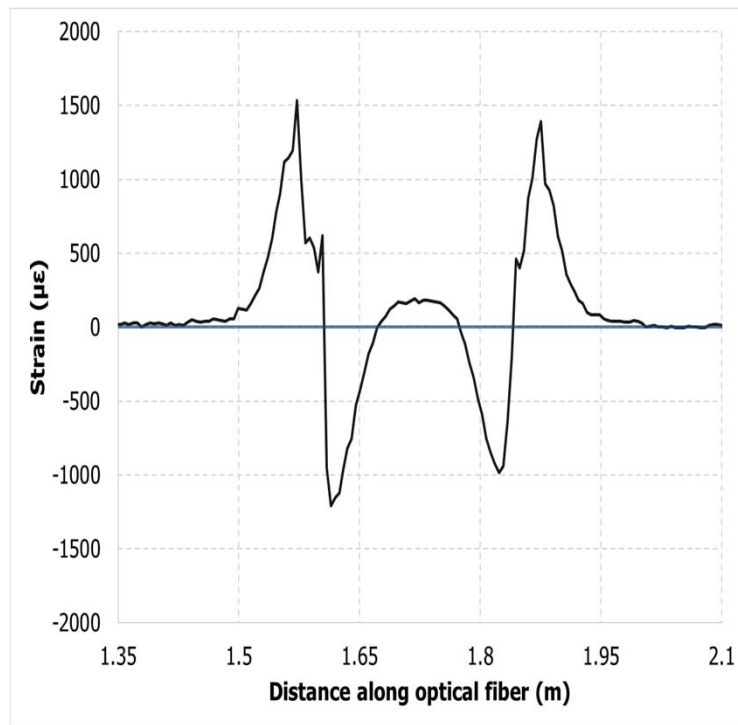


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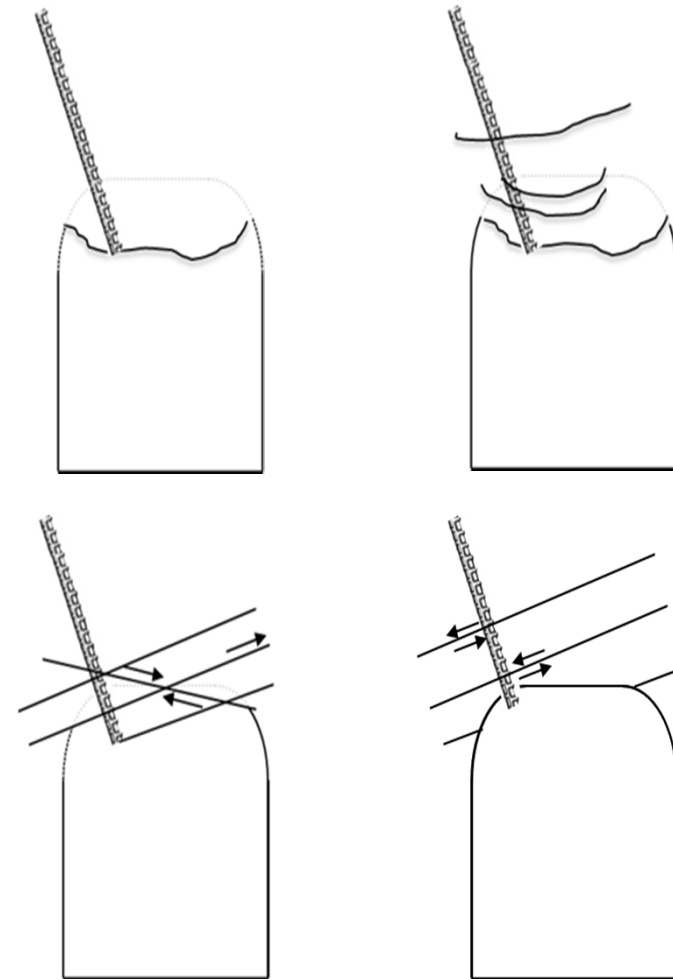
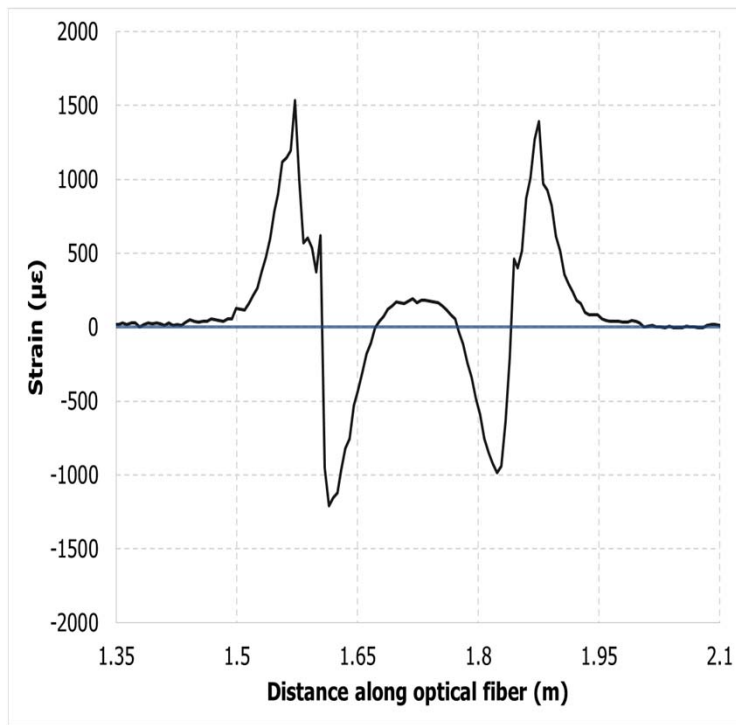
# Is Continuous Monitoring Necessary?

1.25mm



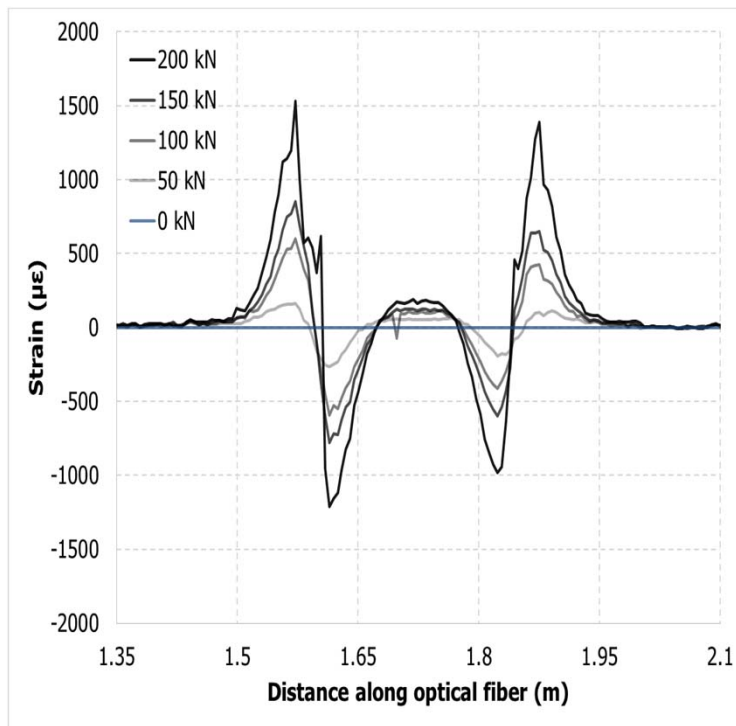
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# Is Continuous Monitoring Necessary?

1.25mm



Double shear test



# Field Trials



Depth of ore body approx. 725ft



3-groove DOS instrumented bolt  
installed in shale roof of a coal mine



# Conclusions

- DOS provides a novel opportunity in support monitoring:
  - Provides a very sensitive spatial resolution (1.25mm)
  - Has a high operational accuracy
  - Optical fiber is extremely cheap (implement an array of DOS elements)
- Validated for further development with temporary support system(s):
  - improve understanding of individual components of the umbrella arch system and the system as a whole with a view to improving design
  - potential tool for predicting ground conditions ahead of the tunnel face





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Associate Professor  
Civil Engineering Department  
Royal Military College of Canada  
04 October 2015

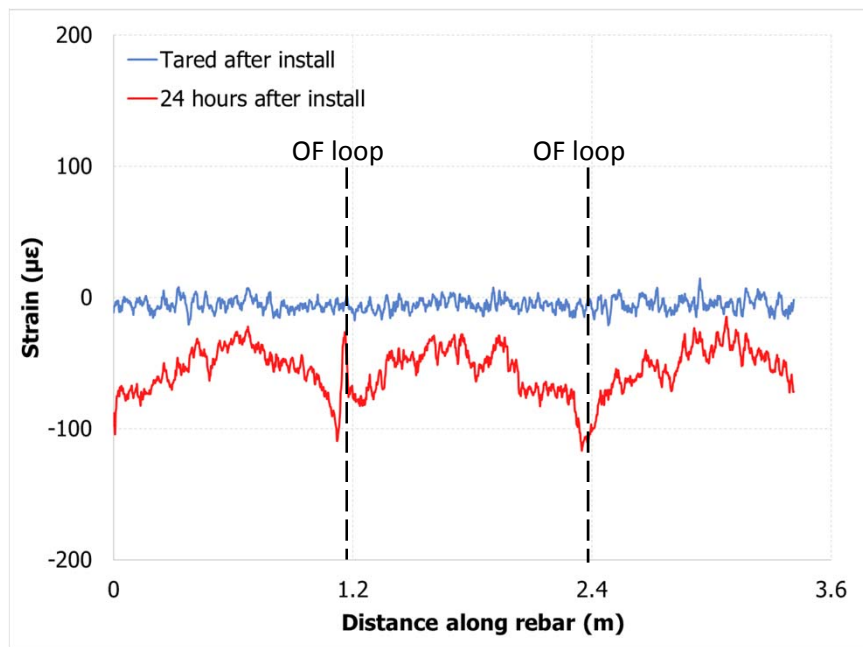
Tunnelling Association of Canada  
Association Canadienne Des Tunnels



# Research Support



# Field Trials



Depth of ore body approx. 725ft



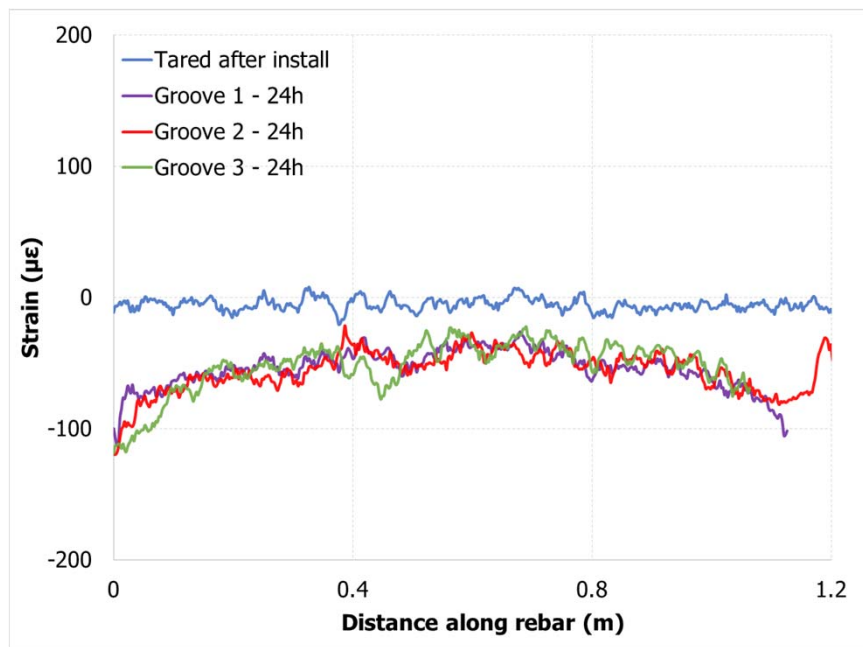
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Tunnelling Association of Canada (TAC) – Ontario Chapter

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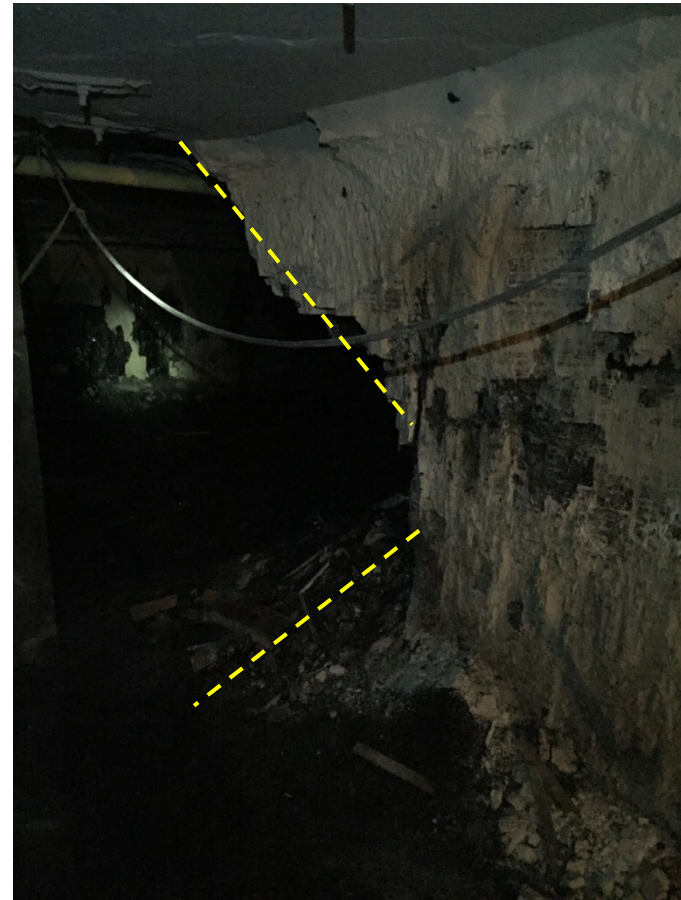
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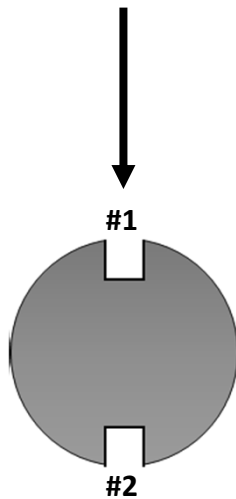
# Orientation of bending

- Developed a DOS rebar element with three lengths of optical fiber (i.e. three machined out grooves)
- Fiber lengths are orientated at approx. 120 deg. from each other
- Determine the direction of maximum bending, in turn allowing the magnitude of bending to be calculated, and ultimately the axial component separate from bending
- Signal loss overcome at loops by using a specialized low-bend loss optical fiber

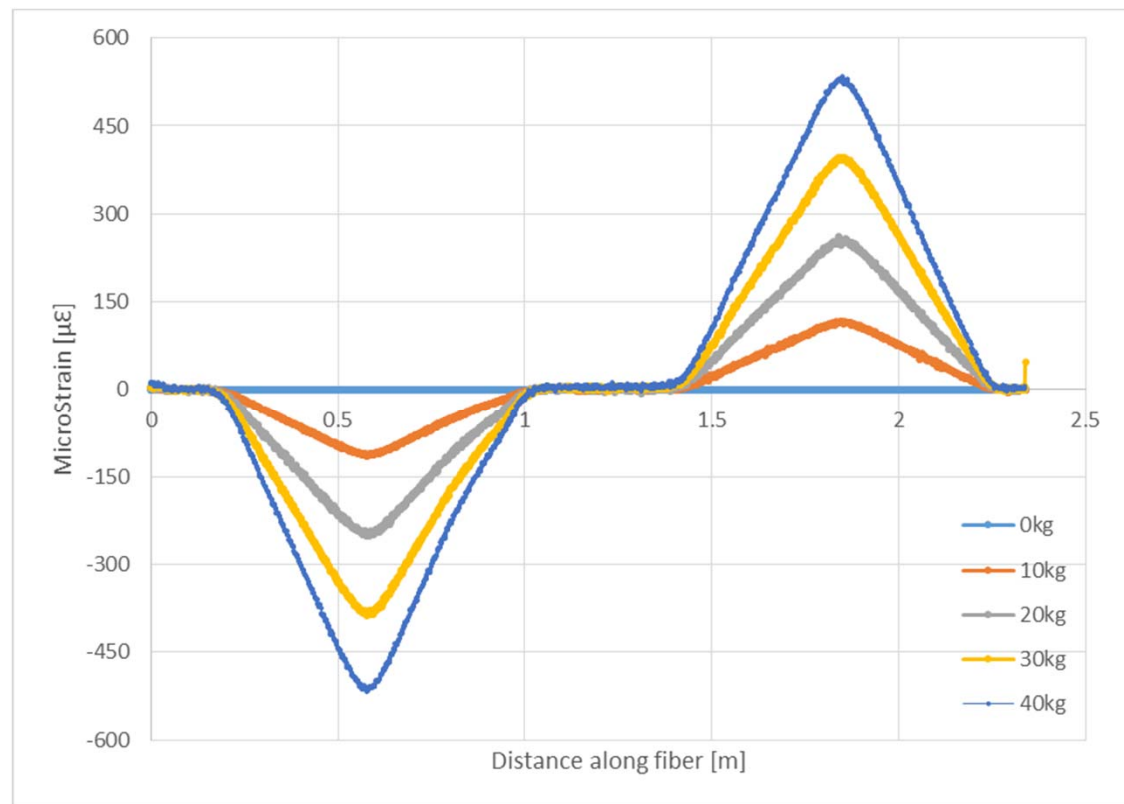


# Orientation of bending

Why we need 3 grooves!

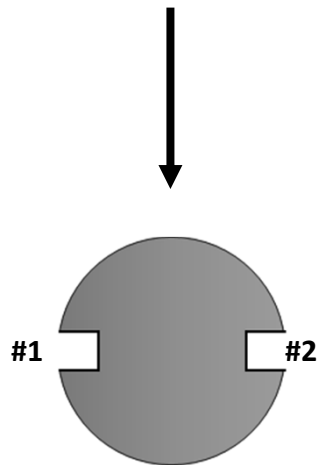


- Two grooves aligned with bending direction

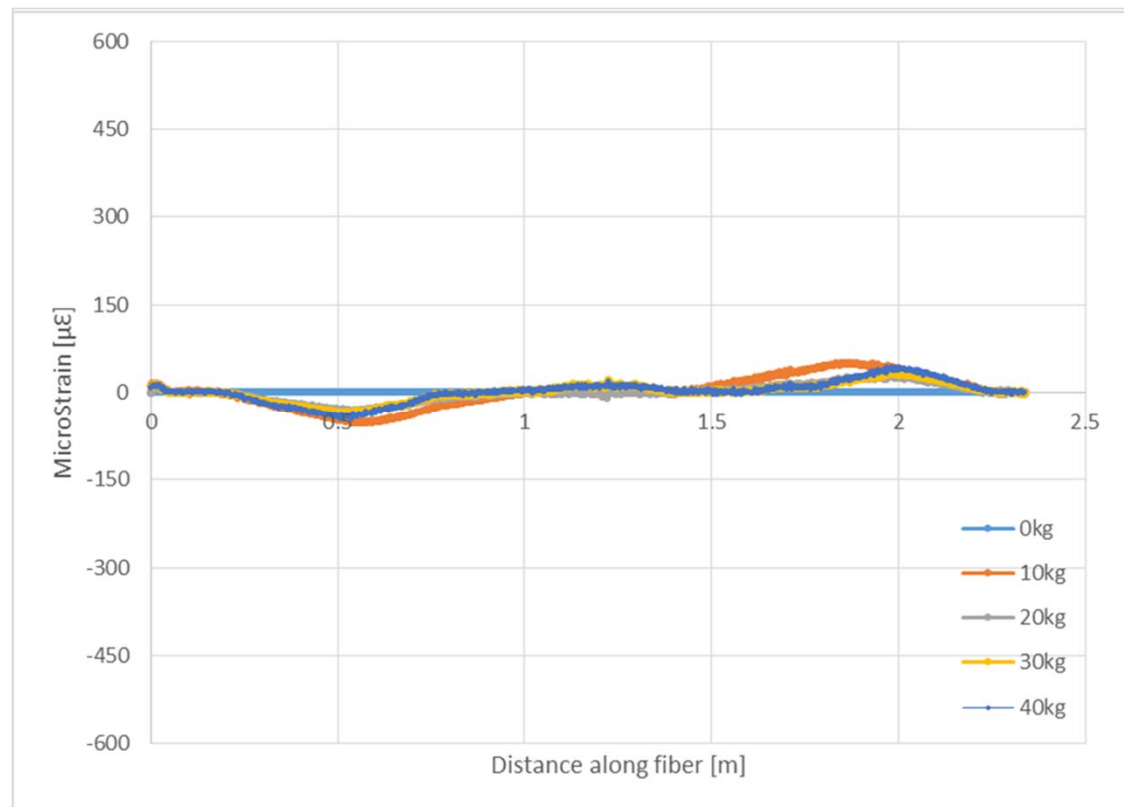


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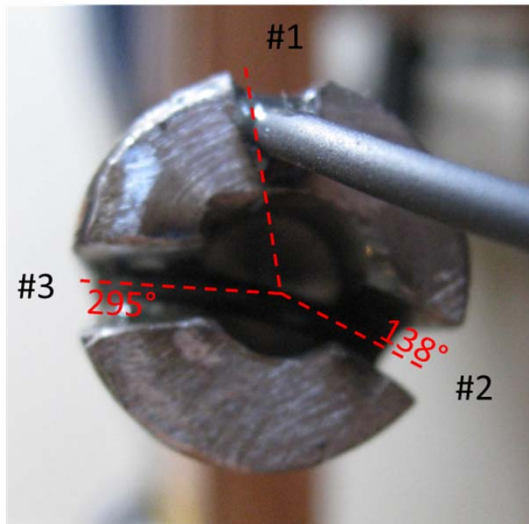
- Two grooves aligned 90 deg. from bending direction (i.e. neutral axis)





# Orientation of bending

- $\theta$ : Orientation (degrees) of maximum bending from length 1  
 1  
 $\varphi$ : Orientation (degrees) of specified length from length 1



$$\varepsilon_{Total}^i = \varepsilon_{axial}^i + \varepsilon_{bending}^i$$

$$\varepsilon_{bending}^i = \varepsilon_{bending} \cos(\theta + \varphi)$$

$$\varepsilon_{Total}^i = \varepsilon_{axial}^i + \varepsilon_{bending} \cos(\theta + \varphi^i)$$

$$E = \left\{ \frac{\varepsilon_{Total}^1 - \varepsilon_{Total}^3}{\varepsilon_{Total}^1 - \varepsilon_{Total}^2} \right\}$$

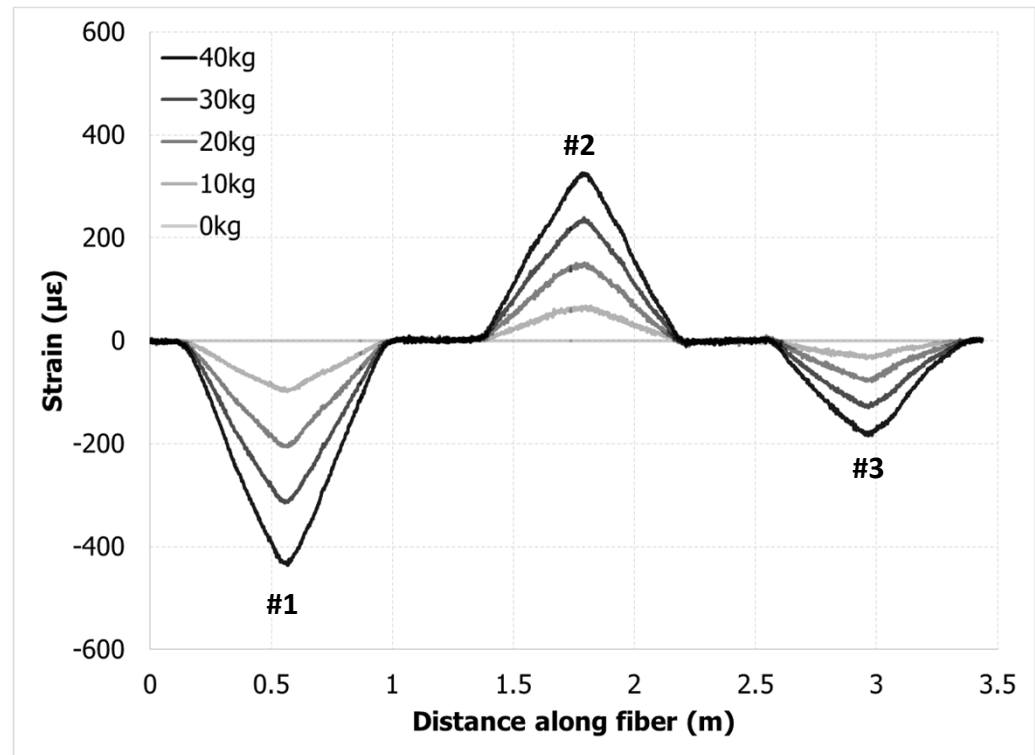
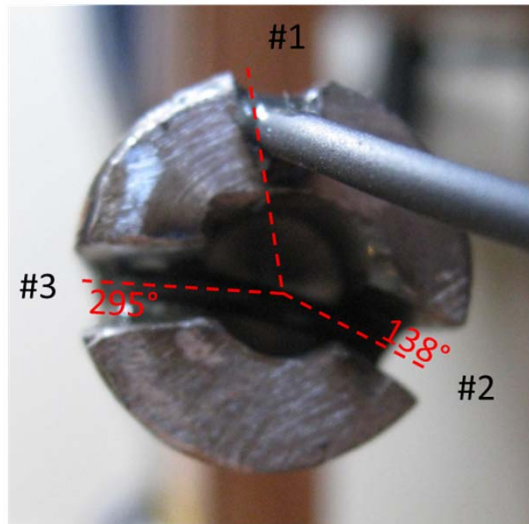
$$= \left\{ \frac{\cos(\theta) - \cos(\theta) \cos(\varphi^3) - \sin(\theta) \sin(\varphi^3)}{\cos(\theta) - \cos(\theta) \cos(\varphi^2) - \sin(\theta) \sin(\varphi^2)} \right\}$$

$$\sin(\varphi^3) \tan(\theta) - E \sin(\varphi^2) \tan(\theta) = 1 - \cos(\varphi^3) - E + E \cos(\varphi^2)$$

$$\theta = \tan^{-1} \left\{ \frac{1 - \cos(\varphi^3) - E + E \cos(\varphi^2)}{\sin(\varphi^3) - E \sin(\varphi^2)} \right\}$$



# Orientation of bending



# Orientation of bending

